

_CAP ₺½x6x0'-6"

STEEL COLUMN

REFER PLAN

TYP. PARAPET GIRT TO COLUMN

*WELD AS REQUIRED BY

JOIST MANUFACTURER

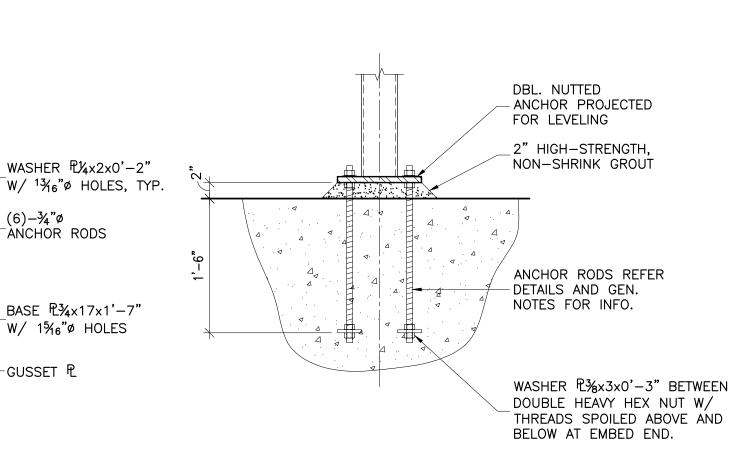
HSS PARAPET GIRTS

CORNER COLUMNS)

T.O. STEEL
ELEV.=REFER PLAN

(USE MITER CUTS AT-

SCALE: NONE



BASE P3/x10x1'-4"

WASHER $\frac{9}{4}$ x2x0'-2"

 $W/ \frac{13}{6}$ W/ HOLES, TYP.

(6)-¾"ø

 $-\mathsf{GUSSET}$ $extstyle{\mathbb{P}}_{\!\!\!\!-}$

ANCHOR RODS

W/1%6 MOLES

W/ 15/16" MOLES

3"ø A325-N BOLT SCHEDULE FOR SINGLE PLATE SHEAR TAB CONNECTIONS (FACTORED LOADS)						
END REACTION	NO. OF BOLTS					
0 THRU 24.8 KIPS	2- 3 ø					
24.9 THRU 43.4 KIPS	3— 3 ø					
43.5 THRU 62.5 KIPS	$4 - \frac{3}{4} \phi$					
62.6 THRU 81.3 KIPS	5— 3 ø					
81.4 THRU 100 KIPS	6— <mark>3</mark> ø					
101 THRU 118 KIPS	$7 - \frac{3}{4} \phi$					
119 THRU 137 KIPS	8- 3 ø					

- 1. VALUES SHOWN ARE APPLICABLE FOR SINGLE PLATE SHEAR TAB CONNECTIONS
- 2. SEE PLAN FOR END REACTIONS
- 3. L = PLATE LENGTH \geq T/2 OF CONNECTED BEAM.
- 4. AT HSS OR PIPE COLUMNS, A THROUGH-PLATE WITH EQUAL WELD ON THE BACK SIDE OF THE COLUMN IS REQUIRED UNDER EITHER OF THE FOLLOWING CIRCUMSTANCES:
- A. FOR SQUARE OR RECTANGULAR HSS:
- WHEN $\frac{B-2.79t}{0.93t} > 35.1$ B. FOR ROUND HSS OR PIPE: WHEN $\frac{D}{+} > \frac{3.190}{-}$
- B = NOMINAL COLUMN WIDTH ACROSS THE COLUMN FACE WITH THE SINGLE PLATE CONNECTION, IN.
- D = OUTSIDE DIAMETER OF ROUND HSS OR PIPE, IN. t = NOMINAL THICKNESS OF COLUMN, IN. Fy = YIELD STRENGTH OF COLUMN, KSI

_	a ≤ 3½"	Leh ≥ 1½"
		2"- 2"- 11%"
	1¾6"x1" SHORT-SLOT HOLES	NO. OF BOLT LINES (SEE SCHEDULE)

BEAM TO BEAM

BEAM TO HSS COLUMN

a ≤ 3½<u>"</u>

COLUMN

SHORT-SLOT

¹¾6"x1"

HOLES

-Leh <u>></u> 1½"

NO. OF BOLT LINES

(SEE SCHEDULE)

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Partnership L.L.C.

201 N. BROADWAY

SUITE 210

MOORE, OK. 73160

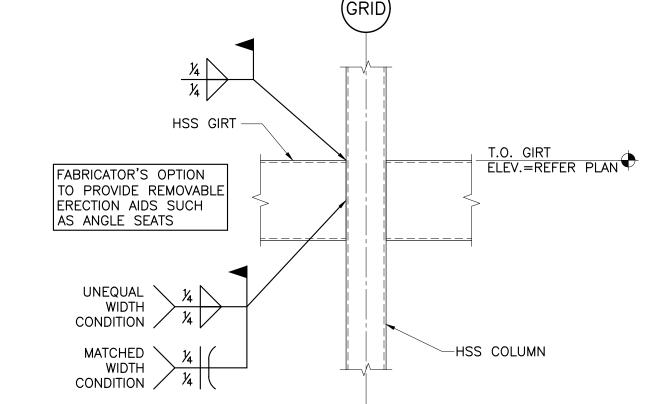
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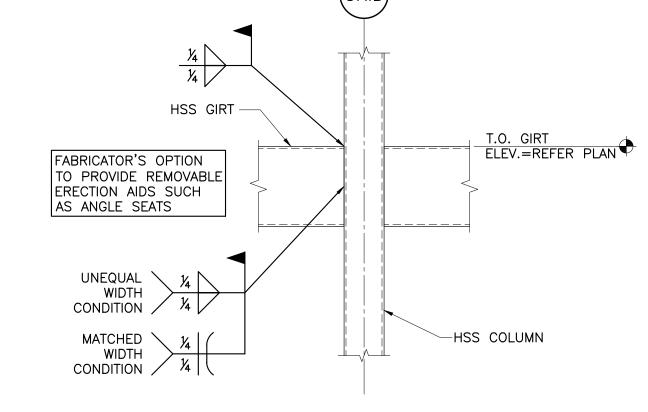
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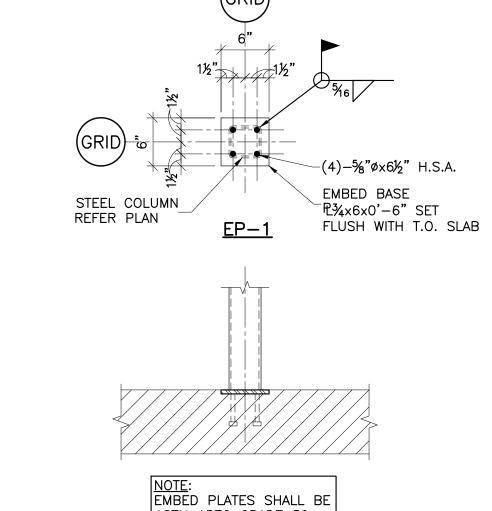


SINGLE PLATE SHEAR TAB CONNECTIONS (LRFD-AISC 14TH EDITION MANUAL) SCALE: NONE



TYP. HSS GIRT TO HSS COLUMN CONNX. SCALE: NONE





BRACING DETAIL FOR STEEL JOISTS W/ POINT LOADS SCALE: NONE

RTU ANGLE FRAME POINT LOAD LOCATION ON TOP STEEL JOISTS OF CHORD —ADD (2) L2x2x¾6 ADD (2) L2x2x3/6_ NOTE: USE THIS DETAIL WHEN POINT LOAD EXCEEDS 50 LBS. AND THE JOIST MANUFACTURER HAS DESIGNED THE JOIST FOR THE FULL CONCENTRATED LOAD. ANGLES TO BE IN PLACE POINT LOAD LOCATION ON BOTTOM OF CHORD PRIOR TO APPLICATION OF LOAD. CONTRACTOR WILL COORDINATE ALL SUPERIMPOSED LOADS WITH THE JOIST MANUFACTURER & ENGINEER PRIOR TO APPLICATION OF LOAD.

> MOORE PUBLIC SCHOOLS **BOARD OF EDUCATION** MOORE, OKLAHOMA

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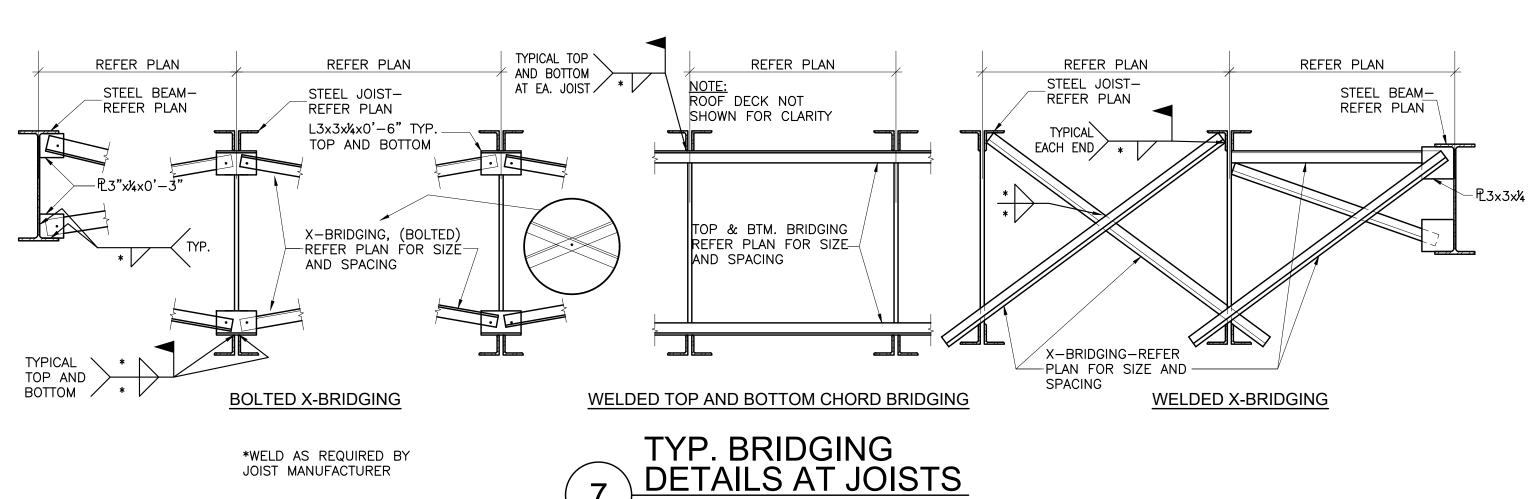
Kirkpatrick Forest Curtis PC
Structural Engineering
OK CA #3888, EXP. 06/30/25
525 Central Park Drive, Suite 202
Oklahoma City, OK 73105

405.528.4596 | kfcengr.com

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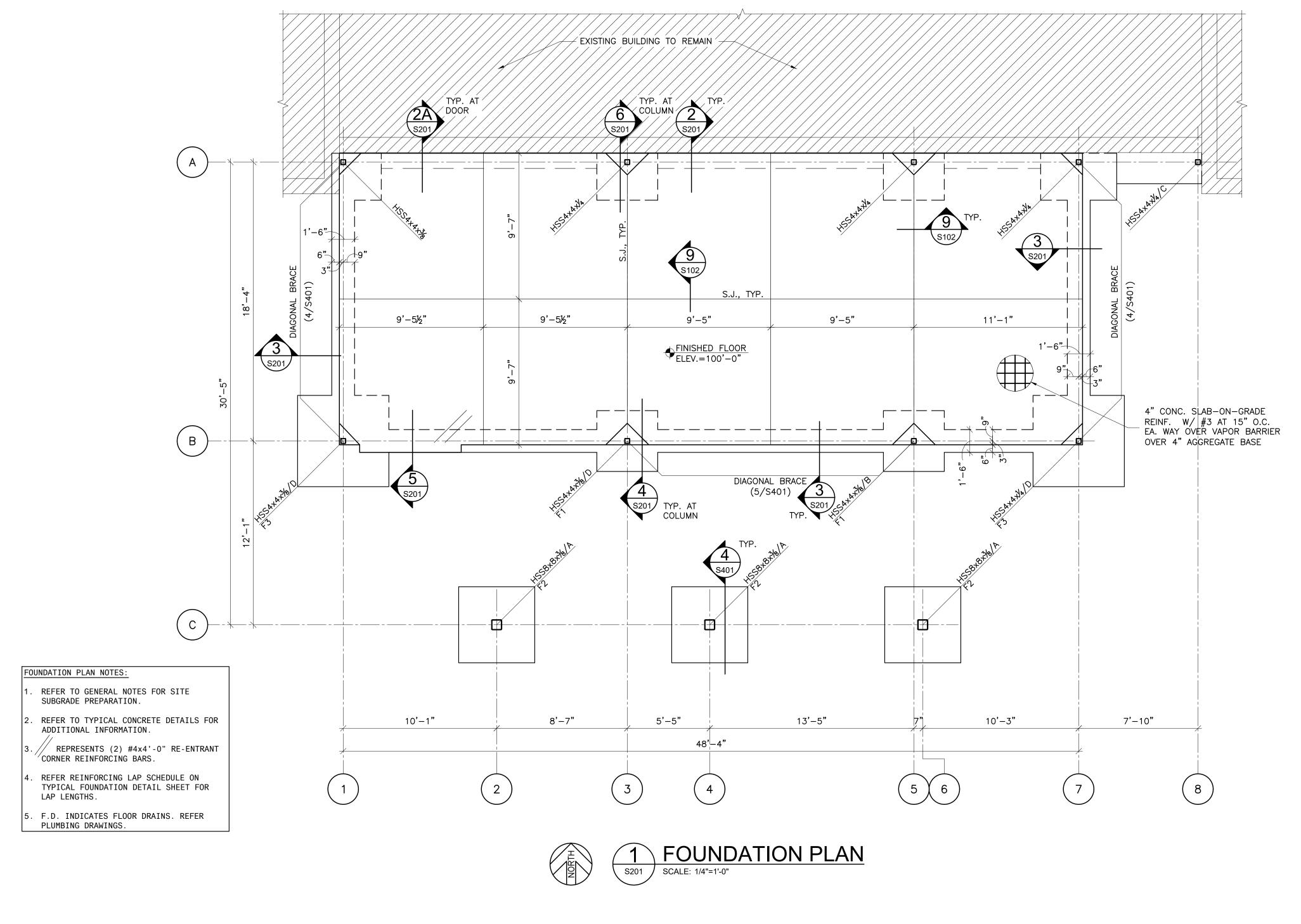
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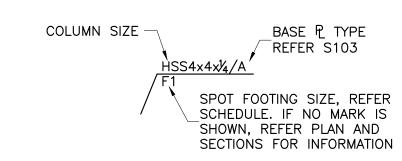
SCALE: NONE

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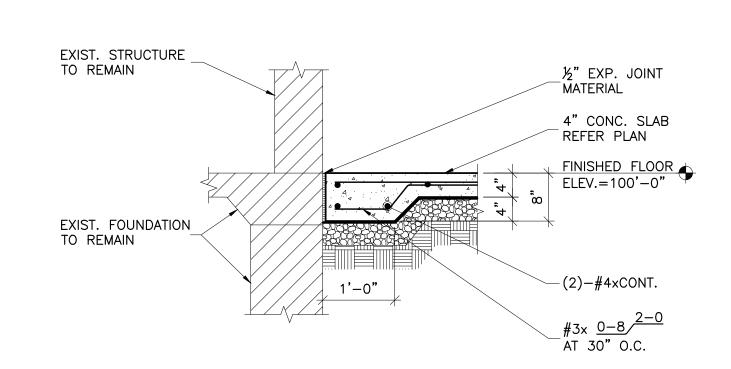
ASTM A572 GRADE 50.



FOUNDATION PLAN LEGEND:

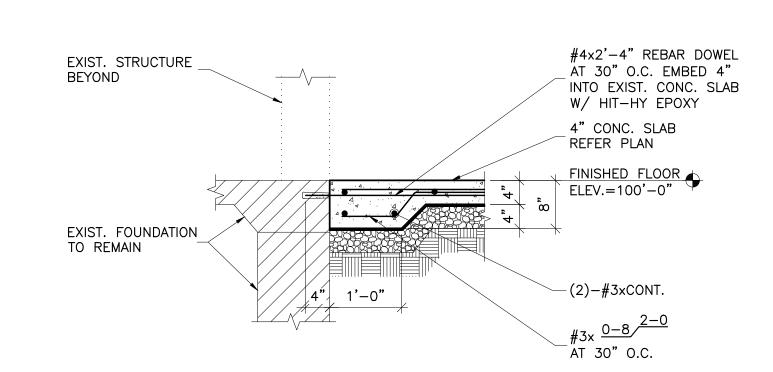


	SPOT FOOTING SCHEDULE				
	MARK	SIZE		REINFORCEMENT	
	MARK	WIDTH	LENGTH	DEPTH	REINFORGERIENT
	F1	4'-0"	4'-0"	2'-0"	(5)-#5 TOP AND
					BOTTOM EA. WAY
	F2 5'-0"	5'-0"	2'-0"	(6)-#5 TOP AND	
		3 -0	3	2 -0	BOTTOM EA. WAY
	F3 6'-0"	6'-0"	2'-0"	(7)-#5 TOP AND	
		0 -0	-0 0 -0	2 -0	BOTTOM EA. WAY



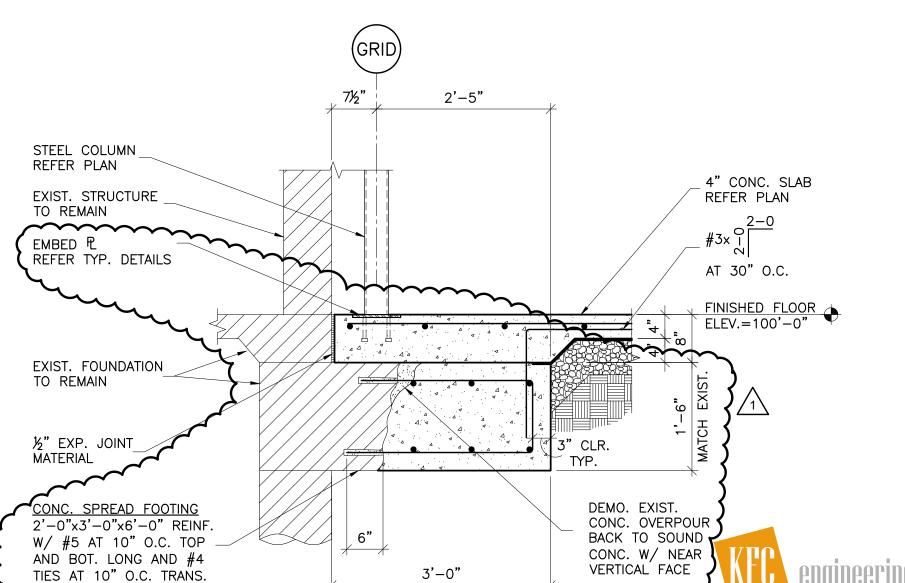
SECTION

SCALE: 3/4"=1'-0"



2A SECTION

S201 / SCALE: 3/4"=1'-0"



SECTION

SCALE: 3/4"=1'-0"

S201

EMBED 6" INTO EXIST. CONC

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> 201 N. BROADWAY SUITE 210 MOORE, OK. 73160 405.735.3477 AGP@theAGP.net www.theAGP.net

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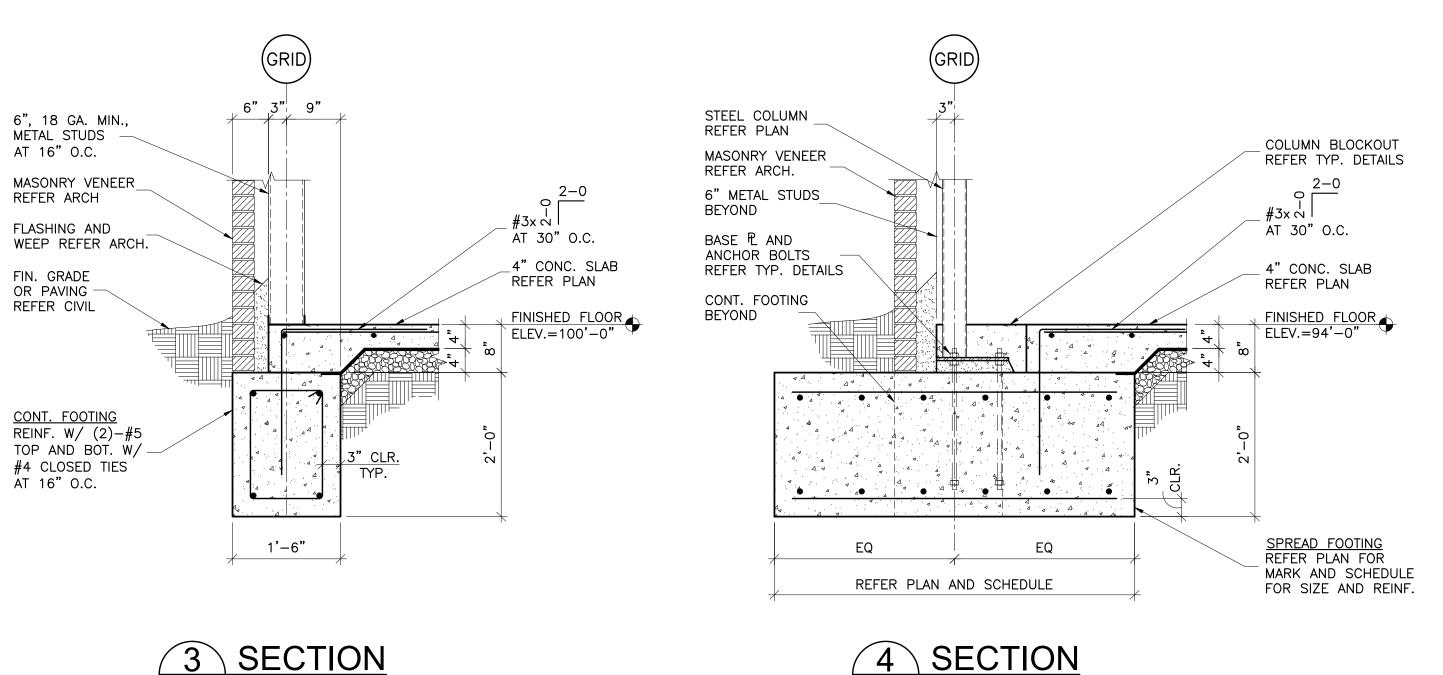
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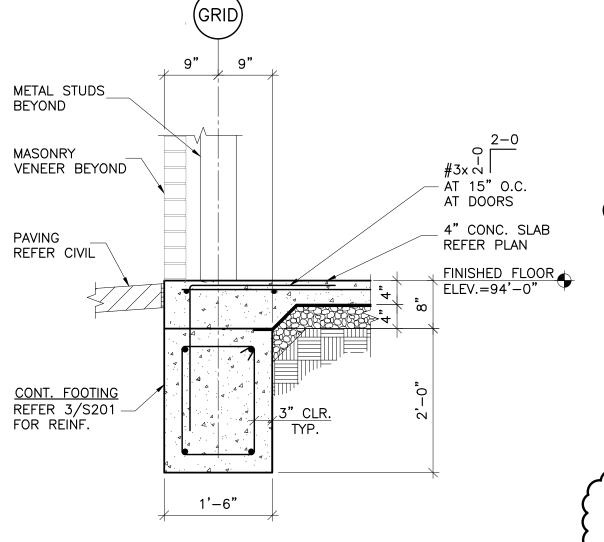
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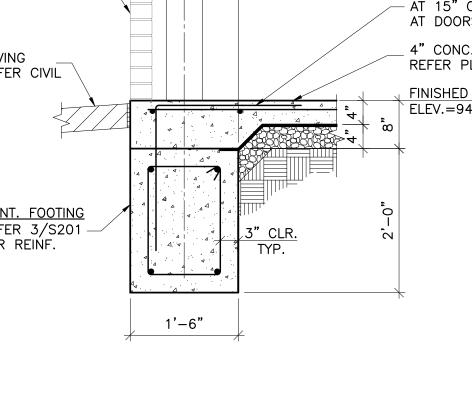


SCALE: 3/4"=1'-0"

S201

SCALE: 3/4"=1'-0"





SECTION SCALE: 3/4"=1'-0"