MOORE PUBLIC SCHOOLS **BOARD OF EDUCATION**

MOORE PUBLIC SCHOOLS DISTRICT NO. I-2 CLEVELAND COUNTY MOORE, OKLAHOMA

CLASSROOM ADDITION HIGHLAND WEST JUNIOR HIGH SCHOOL

901 NORTH SANTA FE MOORE, OK. 73160



the Abla Griffin Partnership



201 N. BROADWAY SUITE 210 MOORE, OK. 73160 405.735.3477 AGP@theAGP.net www.the AGP.net

CONSTRUCTION MANAGER



1909 S. EASTERN AVE. MOORE, OK 73160

MECHANICAL/ELECTRICAL/PLUMBING

SALAS O'BRIEN

2600 VAN BUREN ST., SUITE 2604 NORMAN, OKLAHOMA 73072

STRUCTURAL

KFC ENGINEERING

205 NW 63rd, SUITE 390 OKLAHOMA CITY, OK 73116

INDEX TO DRAWINGS



TOPOGRAPHIC SURVEY - FOR INFORMATION ONLY G100 LEGENDS / MAPS / ETC. C1.00 DEMOLITION PLAN C2.00 SITE PLAN PAVING / GRADING PLAN /1\ C3.03 DETENTION POND PLAN UTILITY PLAN C5.00 **EROSION CONTROL PLAN** C5.01 **EROSION CONTROL DETAILS** C6.00 STANDARD DETAILS C6.01 STANDARD DETAILS C700 SIDEWALK DETAILS DETENTION POND PLANS **DETENTION COVER** GRADING PLAN **DRAINAGE - HISTORIC** C3.02 DRAINAGE - DEVELOPED C3.03 DETENTION POND PLAN C5.00 **EROSION CONTROL PLANS** C5.01 **EROSION CONTROL DETAILS** S100 GENERAL NOTES S101 GENERAL NOTES S102 GENERAL NOTES S103 DETAILS S104 DETAILS S105 DETAILS S106 SPECIAL INSPECTIONS S200 **OVERALL FOUNDATION PLAN** S201 FOUNDATION PLAN - AREA "A" S202 FOUNDATION PLAN - AREA "B" S203 FOUNDATION PLAN - AREA "C" S204 FOUNDATION PLAN - AREA "D" S300 OVERALL FRAMING PLAN S301 FRAMING PLAN - AREA "A" S302 FRAMING PLAN - AREA "B" S303 FRAMING PLAN - AREA "C" S304 FRAMING PLAN - AREA "D" S501 FOUNDATION SECTIONS S601 FRAMING SECTIONS S602 FRAMING SECTIONS S603 FRAMING SECTIONS A100 LIFE SAFETY PLAN DEMOLITION PLAN A101 OVERALL FLOOR PLAN - CLASSROOM AREA / OVERALL FLOOR PLAN - OFFIC A102 A103 FLOOR PLAN - AREA "A" A104 FLOOR PLAN - AREA "B" A105 FLOOR PLAN - AREA "C" A106 FLOOR PLAN - AREA "D" / MILLWORK ROOM #29 A107 EXTERIOR PLAN - MAIN BUILDING A108 EXTERIOR PLAN - EXISTING GYM - ALTERNATE #1 A109 ENLARGED FLOOR PLANS / INTERIOR ELEVATIONS REFLECTED CEILING PLAN - AREA "A" A110 A111 REFLECTED CEILING PLAN - AREA "B" REFLECTED CEILING PLAN - AREA "C" A112 A113 REFLECTED CEILING PLAN - AREA "D" A114 ROOF PLAN - CLASSROOM AREA / ROOF PLAN - OFFICE AREA **ROOF DETAILS** A115 A116 LVT & CARPET DIM. / DESIGN PLAN - AREA "A" A117 LVT & CARPET DIM. / DESIGN PLAN - AREA "B" A118 LVT & CARPET DIM. / DESIGN PLAN - AREA "C" A119 LVT & CARPET DIM. / DESIGN PLAN - AREA "D" A120 EQUIPMENT FLOOR PLAN - AREA "A" / EQUIPMENT SCHEDULE A121 EQUIPMENT FLOOR PLAN - AREA "B" A122 EQUIPMENT FLOOR PLAN - AREA "C" A123 EQUIPMENT FLOOR PLAN - AREA "D" A124 ENLARGED FLR PLAN - TYPICAL LAB EQUIP. & STORAGE ROOM / ENLARGED A201 **BUILDING ELEVATIONS DEMOLITION** A201a **BUILDING ELEVATIONS** A202 BUILDING ELEVATIONS DEMOLITION A202a **BUILDING ELEVATIONS** A203 **BUILDING ELEVATIONS - ALTERNATE #1** A301 **BUILDING SECTIONS** A302 BUILDING SECTION A303 WALL SECTIONS / DETAILS A304 WALL SECTIONS / DETAILS A305 WALL SECTIONS / DETAILS

SHEET NUMBER

С

DESCRIPTION

COVER SHEET

MICHAEL L ABLA 2639

CEDAR CREEK

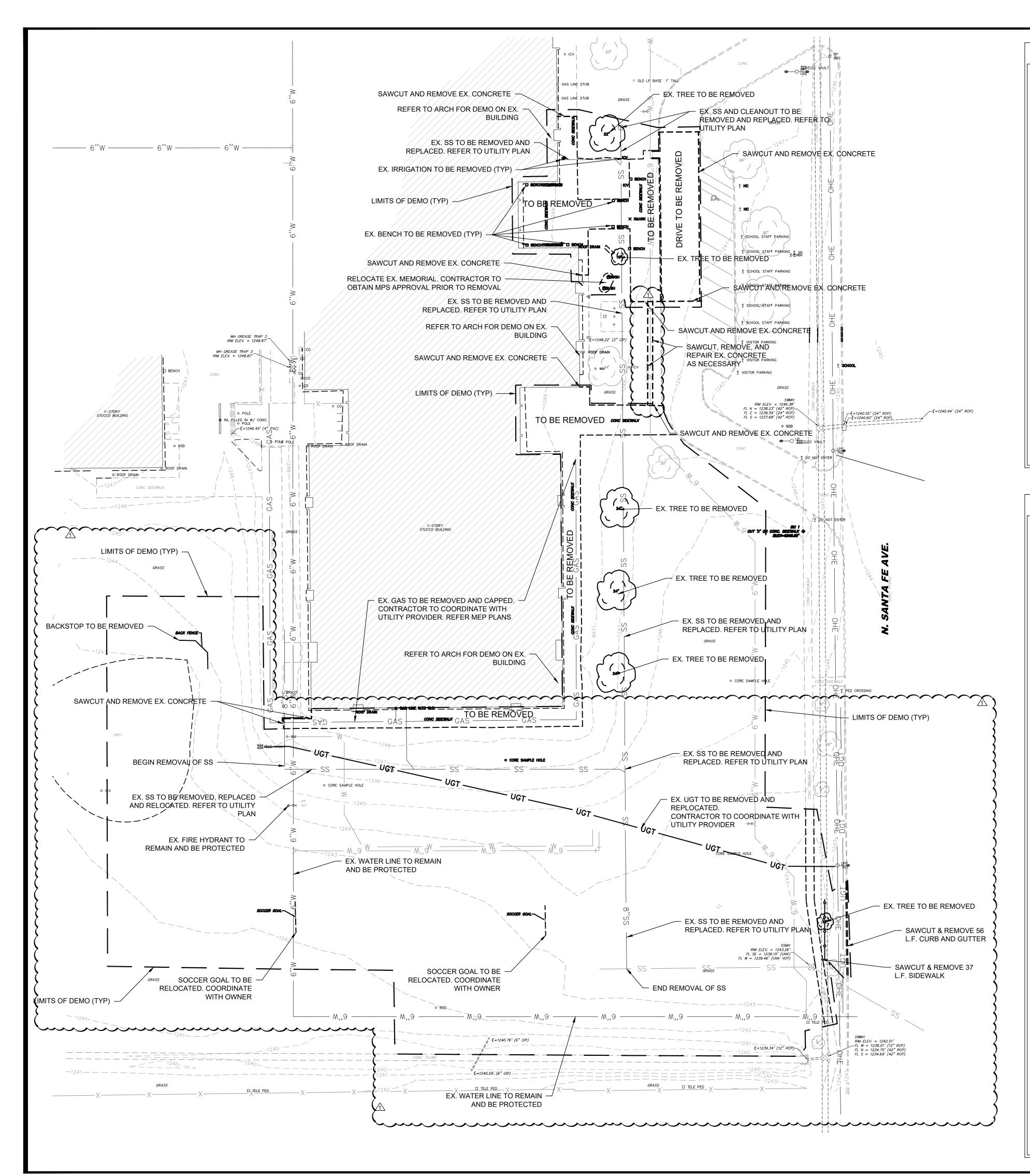
CIVIL

11912 N. PENNSYLVANIA AVE., SUITE D4

OKLAHOMA CITY, OK 73120

	SHEET NUMBER	DESCRIPTION	
	A401	INTERIOR ELEVATIONS	
	A501	DETAILS	
	A601	ROOM FINISH SCHEDULE / COLOR SCHEDULE	
	A602	DOOR SCHEDULE / DOOR ELEVATIONS / FRAME ELEVATIONS	
	A701 A702	MILLWORK DETAILS MILLWORK DETAILS	
	A703	MILLWORK DETAILS	
	F000	FIRE SPRINKLER RISER DIAGRAM / NOTES	
	F001	FIRE PROTECTION PLAN - SITE	
	F101	FIRE PROTECTION PLAN - AREA "A"	
	F102	FIRE PROTECTION PLAN - AREA "B"	
	F103 F104	FIRE PROTECTION PLAN - AREA "C" FIRE PROTECTION PLAN - AREA "D"	
	P000	GENERAL PLUMBING NOTES	
	PD101	PLUMBING DEMOLITION PLAN - OVERALL	
	P001	PLUMBING PLAN - SITE	
	P101	PLUMBING PLAN - BELOW GRADE - AREA "A"	
	P102	PLUMBING PLAN - BELOW GRADE - AREA "B"	
	P103 P104	PLUMBING PLAN - BELOW GRADE - AREA "C" PLUMBING PLAN - BELOW GRADE - AREA "D"	
	P110	PLUMBING PLAN - ABOVE GRADE - AREA "A"	
	P111	PLUMBING PLAN - ABOVE GRADE - AREA "B"	
	P112	PLUMBING PLAN - ABOVE GRADE - AREA "C"	
	P113	PLUMBING PLAN - ABOVE GRADE - AREA "D"	
	P201	PLUMBING PLAN - ROOF	
	P301 P302	PLUMBING ISOMETRIC WASTE & VENT PLUMBING ISOMETRIC WATER SUPPLY	
	P501	DETAILS	
	P601	SCHEDULES	
	M000	MECHANICAL NOTES	
	M001	MECHANICAL SITE PLAN	
	M002	MECH. OVERALL PLAN - CLAQSSROOM AREA / MECH. OVERALL PLAN - OFFIC	CE AREA
	M101 M102	MECHANICAL PLAN - AREA "A" MECHANICAL PLAN - AREA "B"	
	M103	MECHANICAL PLAN - AREA "C"	
	M104	MECHANICAL PLAN - AREA "D"	
	M110	MECHANICAL OVERALL ROOF PLAN	
	M111	MECH. ROOF PLAN - CLASSROOM AREA / MECH. ROOF PLAN - OFFICE AREA	
	M501 M502	DETAILS DETAILS	
	M601	SCHEDULES	
	Т000	TECHNOLOGY LEGENDS / NOTES	
	T100	TECHNOLOGY SITE PLAN	
	T101	TECHNOLOGY SITE PLAN ENLARGED	
CE AREA	T201 T202	TECHNOLOGY PLAN - AREA A TECHNOLOGY PLAN - AREA B	
	T203	TECHNOLOGY PLAN - AREA C	
	T204	TECHNOLOGY PLAN - AREA D	
	T301	IDF ROOM - I.T. 9	
	T401	DETAILS	
	T402 T403	DETAILS DETAILS	
	T403	DETAILS	
	T501	SYSTEM SPECIFICATIONS	
	T502	SYSTEM SPECIFICATIONS	
	T503	SYSTEM SPECIFICATIONS	
	T504 E000	SYSTEM SPECIFICATIONS ELECTRICAL NOTES / SCHEDULES	
	E000 E001	ELECTRICAL NOTES / SCHEDULES ELECTRICAL SITE PLAN	
	E002	OVERALL LIGHTING PLAN - CLASSROOM AREA / OVERALL LIGHTING PLAN - C	OFFICE
	E101	ELECTRICAL LIGHTING PLAN AREA "A"	
	E102	ELECTRICAL LIGHTING PLAN AREA "B"	
	E103	ELECTRICAL LIGHTING PLAN AREA "C"	
	E104 E200	ELECTRICAL LIGHTING PLAN AREA "D" OVERALL POWER PLAN - CLASSROOM AREA / OVERALL POWER PLAN - OFFI	CE
	E201	ELECTRICAL POWER \PLAN AREA "A"	02
D PLAN ROOM #16	E202	ELECTRICAL POWER PLAN AREA "B"	
	E203	ELECTRICAL POWER \PLAN AREA "C"	
	E204		
	E210 E211	ELECTRICAL POWER PLAN - SITE ROOF 6 표 OVERALL POWER PLAN - CLASSROOM AREA / 표 실	
		ELECTRICAL POWER PLAN AREA "D" ELECTRICAL POWER PLAN - SITE ROOF OVERALL POWER PLAN - CLASSROOM AREA / OVERALL POWER PLAN - OFFICE ELECTRICAL ONE-LINE DIAGRAM / SCHEDUILE	_
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		DETAILS SCHEDULES	Ш S

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	date:
	JULY 2023



LEGEND				
BOUNDARY LINE RIGHT OF WAY LINE EASEMENT LINE EESISTING CONCRETE CURB AND GUTTER PROPOSED CONCRETE CURB AND GUTTER PROPOSED FIRE LANE STRIPING OHE OVERHEAD ELECTRIC UGE UNDERGROUND ELECTRIC GAS GAS LINE UGT UNDERGROUND TELEPHONE FO UNDERGROUND FIBER OPTIC SS SANITARY SEWER WATERLINE BENCHMARK				
→ FIRE HYDRANT WATER VALVE	Ø Ø TPD	EX. POWER POLE PROP. POWER POLE		
		EX. TELEPHONE PED.		
 EX. WATER METER PROP. WATER METER 		EX. TELEPHONE MANHOLE		
© ^{SCV} EX. SPRINKLER VALVE		EX. TRAFFIC CONTROL BO		
	O O	EX. FLAG POLE		
$\Box^{EPD} = EX. ELECT. PEDESTAL$	Ø	EX. YARD LIGHT		
	-	EX. GREASE TRAP		
EX. ELECT. METER	S	EX. SS MANHOLE		
	65	PROP. SS MANHOLE		
\square^{AC} EX. AIR CONDITIONER	Ø	EX. GAS METER		
EX. SIGNAGE		PROP. GAS METER		
℁ EX. LIGHT POLE	P	EX. ELECT. MANHOLE		
	S	EX. STORM MANHOLE		
O ^{GP} EX. BOLLARD				

- THE CONTRACTOR SHALL ABIDE BY ALL FEDERAL, STATE AND LOCAL CODES FOR THE DEMOLITION AND DISPOSAL OF ALL MATERIALS.
- CEDAR CREEK CONSULTING, INC. SHALL NOT BE LIABLE FOR ANY DEMOLITION PROCEDURES, SCHEDULING, AND DISPOSING OF ANY MATERIALS.
- IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO MAKE SURE THAT THE PROPERTY IS NOT DAMAGED AND IS ACCESSIBLE AT ALL TIMES, AND THAT CONSTRUCTION DOES NOT CREATE ANY HARDSHIP TO LAND OWNERS.
- 4. THE CONTRACTOR IS RESPONSIBLE FOR THE DEMOLITION, REMOVAL, AND DISPOSING IN A LOCATION APPROVED BY ALL GOVERNING AUTHORITIES, OF ALL STRUCTURES, PADS, WALLS, FLUMES, FOUNDATIONS, PARKING, DRIVES, DRAINAGE, UTILITIES, ETC., SUCH THAT THE IMPROVEMENTS SHOWN ON THE REMAINING PLANS CAN BE CONSTRUCTED. ALL FACILITIES TO BE REMOVED SHALL BE UNDERCUT TO SUITABLE MATERIAL AND BROUGHT TO GRADE WITH SUITABLE COMPACTED FILL MATERIAL PER THE SPECIFICATIONS.
- THE CONTRACTOR SHALL COORDINATE WITH RESPECTIVE UTILITY COMPANIES PRIOR TO THE DISCONNECTION, REMOVAL AND RELOCATION OF ALL UTILITIES. THE CONTRACTOR SHALL COORDINATE WITH THE UTILITY COMPANY CONCERNING PORTIONS OF WORK WHICH MAY BE PERFORMED BY THE UTILITY COMPANY'S FORCES AND ANY FEES WHICH ARE TO BE PAID TO THE UTILITY COMPANY FOR SERVICES. THE CONTRACT IS RESPONSIBLE FOR PAYING ALL FEES AND CHARGES.
- THE CONTRACTOR IS RESPONSIBLE FOR REMOVING EXISTING IRRIGATION SYSTEM IN THE AREAS OF PROPOSED IMPROVEMENTS. THE CONTRACTOR SHALL CAP THE EXISTING IRRIGATION SYSTEM TO REMAIN SUCH THAT THE REMAINING SYSTEM SHALL CONTINUE TO FUNCTION PROPERLY.
- THE LOCATIONS OF ALL EXISTING UTILITIES SHOWN ON THIS PLAN HAVE BEEN DETERMINED FROM THE BEST INFORMATION AVAILABLE AND ARE GIVEN FOR THE CONVENIENCE OF THE CONTRACTOR. THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THEIR ACCURACY. PRIOR TO THE START OF ANY DEMOLITION ACTIVITY, THE CONTRACTOR SHALL NOTIFY THE UTILITY COMPANIES FOR ONSITE LOCATIONS OF EXISTING UTILITIES.
- ALL EXISTING SEWERS, PIPING AND UTILITIES SHOWN ARE NOT TO BE INTERPRETED AS THE EXACT LOCATION, OR AS THE ONLY OBSTACLES THAT MAY OCCUR ON THE SITE. VERIFY EXISTING CONDITIONS AND PROCEED WITH CAUTION AROUND ANY ANTICIPATED FEATURES. GIVE NOTICE TO ALL UTILITY COMPANIES REGARDING DESTRUCTION AND REMOVAL OF ALL SERVICE LINES AND CAP ALL LINES BEFORE PROCEEDING WITH WORK. UTILITIES DETERMINED TO BE ABANDONED AND LEFT IN PLACE SHALL BE GROUTED IF UNDER BUILDINGS.
- ELECTRICAL, TELEPHONE, CABLE, WATER, FIBER OPTIC CABLE AND/OR GAS LINES NEEDING TO BE REMOVED OR RELOCATED SHALL BE COORDINATED WITH THE AFFECTED UTILITY COMPANY. ADEQUATE TIME SHALL BE PROVIDED FOR RELOCATION AND CLOSE COORDINATION WITH THE UTILITY COMPANY IS NECESSARY TO PROVIDE A SMOOTH TRANSITION IN UTILITY SERVICE.
- 10. THE CONTRACTOR MUST PROTECT THE PUBLIC AT ALL TIMES WITH FENCING, BARRICADES, ENCLOSURES, ETC.
- 11. ALL EROSION CONTROL DEVICES ARE TO BE INSTALLED PRIOR TO DEMOLITION.

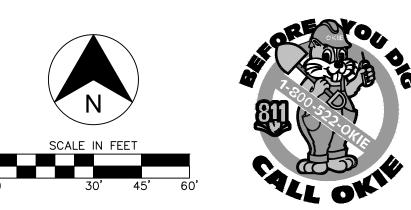
NOTE: CONTRACTOR SHALL ENSURE THAT PEDESTRIAN AND FIRE ACCESS TO THE EXISTING BUILDING IS PROVIDED AT ALL TIMES DURING DEMOLITION AND CONSTRUCTION.

NOTE: CONTRACTOR SHALL BE RESPONSIBLE TO REPAIR ANY DAMAGE TO EXISTING PAVEMENT DUE TO CONSTRUCTION

NOTE: CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING SAFE EGRESS AND INGRESS TO AND FROM SITE ALL ALL TIMES, AS WELL AS ALONG PUBLIC STREETS.

DEMOLITION NOTES

- 12. THE CONTRACTOR MAY LIMIT SAW-CUT AND PAVEMENT REMOVAL TO ONLY THOSE AREAS WHERE IT IS REQUIRED AS SHOWN ON THE CONSTRUCTION PLANS BUT IF ANY DAMAGE IS INCURRED ON ANY OF THE SURROUNDING PAVEMENT, ETC., THE CONTRACTOR SHALL BE RESPONSIBLE FOR ITS REMOVAL AND REPAIR.
- 13. THE CONTRACTOR SHALL MAINTAIN ALL EXISTING PARKING, SIDEWALKS, DRIVES, ETC. CLEAR AND FREE FROM ANY CONSTRUCTION ACTIVITY INCLUDING FENCING AND CONSTRUCTION TRAILER AND/OR MATERIAL TO ENSURE EASY AND SAFE PEDESTRIAN AND VEHICULAR TRAFFIC TO AND FROM THE SITE.
- 14. THE CONTRACTOR SHALL COORDINATE WATERMAIN WORK WITH THE FIRE DEPARTMENT, THE CITY UTILITY DEPARTMENT TO PLAN PROPOSED IMPROVEMENTS AND TO ENSURE ADEQUATE FIRE PROTECTION IS CONSTANTLY AVAILABLE TO THE SITE THROUGHOUT THIS SPECIFIC WORK AND THROUGH ALL PHASES OF CONSTRUCTION. CONTRACTOR WILL BE RESPONSIBLE FOR ARRANGING/PROVIDING ANY REQUIRED WATERMAIN SHUT-OFFS WITH THE CITY/COUNTY DURING CONSTRUCTION. ANY COSTS ASSOCIATED WITH WATERMAIN SHUT-OFFS WILL BE THE RESPONSIBILITY OF THE CONTRACTOR AND NO EXTRA COMPENSATION WILL BE PROVIDED.
- 15. DAMAGE TO ALL EXISTING CONDITIONS TO REMAIN WILL BE REPLACED AT CONTRACTOR'S EXPENSE. REPAIRS SHALL RESTORE DAMAGED ITEMS TO EQUAL OR BETTER THAN EXISTING CONDITIONS. CONTRACTOR IS RESPONSIBLE FOR DOCUMENTING ALL EXISTING DAMAGE AND NOTIFYING THE CONSTRUCTION MANAGER PRIOR TO CONSTRUCTION START.
- 16. ALL TRENCHES AND/OR EXCAVATED AREAS SHALL BE FILLED /TESTED IN ACCORDANCE WITH THE GEOTECHNICAL ENGINEERING REPORT.
- 17. IF SEPTIC TANKS ARE FOUND PRESENT WITHIN THE LIMITS OF DISTURBANCE, THEY SHALL BE DISPOSED OF IN ACCORDANCE WITH LOCAL, STATE AND FEDERAL LAWS.
- 18. IF THE CONTRACTOR FINDS ANY UNDERGROUND TANKS ON SITE, THEY SHALL CONTACT THE ENGINEER IMMEDIATELY.
- 19. ALL WELLS SHALL BE CAPPED AND CLOSED IN ACCORDANCE WITH APPLICABLE STATE AND FEDERAL LAWS.
- 20. ALL ITEMS WITHIN LIMITS OF DEMOLITION SHALL BE REMOVED UNLESS OTHERWISE NOTED.
- 21. CONTRACTOR SHALL BEGIN CONSTRUCTION OF ANY LIGHT POLE BASES FOR RELOCATED LIGHT FIXTURES AND RELOCATION OF ELECTRICAL SYSTEM AS SOON AS DEMOLITION BEGINS. CONTRACTOR SHALL BE AWARE THAT INTERRUPTION OF POWER TO ANY LIGHT POLES OR SIGNS SHALL NOT EXCEED 24 HOURS.
- 22. CONTRACTOR IS TO VERIFY LOCATION OF EXISTING IRRIGATION SYSTEM, VALVE BOXES, CONTROL BOXES, BACKFLOW PREVENTION DEVICES AND LIGHTING CONDUIT. IF DAMAGED THEY MUST BE REPAIRED AT CONTRACTOR'S EXPENSE.



Engineering • Planning • Consulting P.O. Box 14534 Oklahoma City, OK 73113 405-778-3385 www.cedarcreekinc.com OK CA 5864 EXP. 06/30/24 LOCATION MAP: R 3 W SW 119TH ST PROJECT LOCATION SW 134TH ST NOT TO SCALE PROJECT: **HIGHLAND WEST** JR. HIGH

> 901 N. SANTA FE MOORE, OK

PROJECT NUMBER: 23069 DRAWING DATE: 11.02.23 ISSUE DATE: 11.02.23

SEAL:



SUBMITTAL:

PERMIT SET

REVISIONS: 11.02.23

CB #1

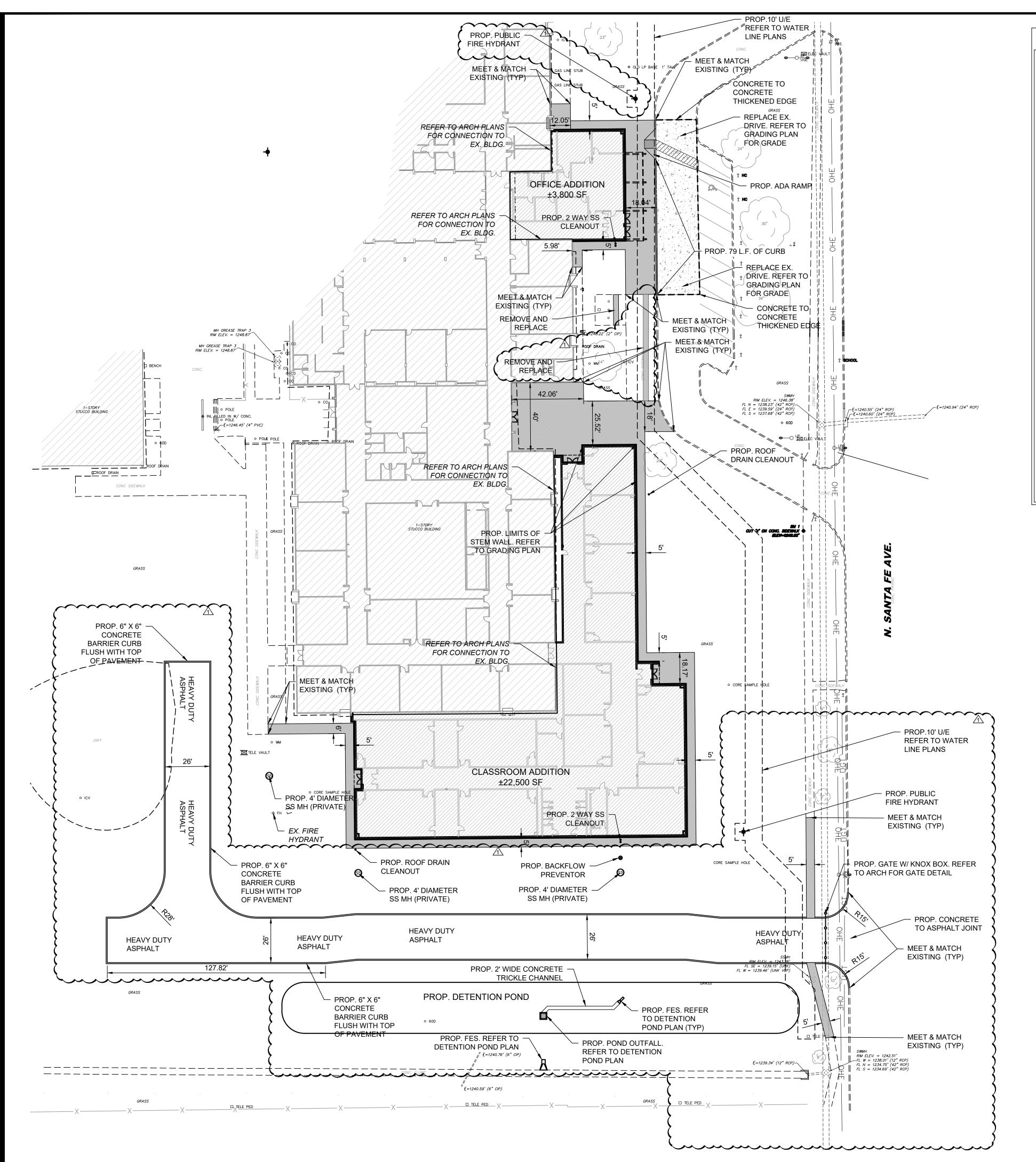
THESE PLANS AND DRAWINGS ARE NOT TO BE REPRODUCED. CHANGED OR COPIED IN ANY FORM OR MANNER WHATSOEVER WITHOUT FIRST OBTAINING THE WRITTEN PERMISSION AND CONSENT F CEDAR CREEK CONSULTING INC. THIS SHEET IS NOT TO BE USED FOR CONSTRUCTION UNLESS THE ISSUE DATE IN THE TITLE BLOCK COINCIDES WITH OR OST DATES THE DRAWING DATE. ANY CHANGES MADE FROM THESE PLANS WITHOUT CONSENT OF CEDAR CREEK CONSULTING INC. ARE UTHORIZED, AND SHALL RELIEVE CEDAR CREEK CONSULTING OF RESPONSIBILITY FOR ALL

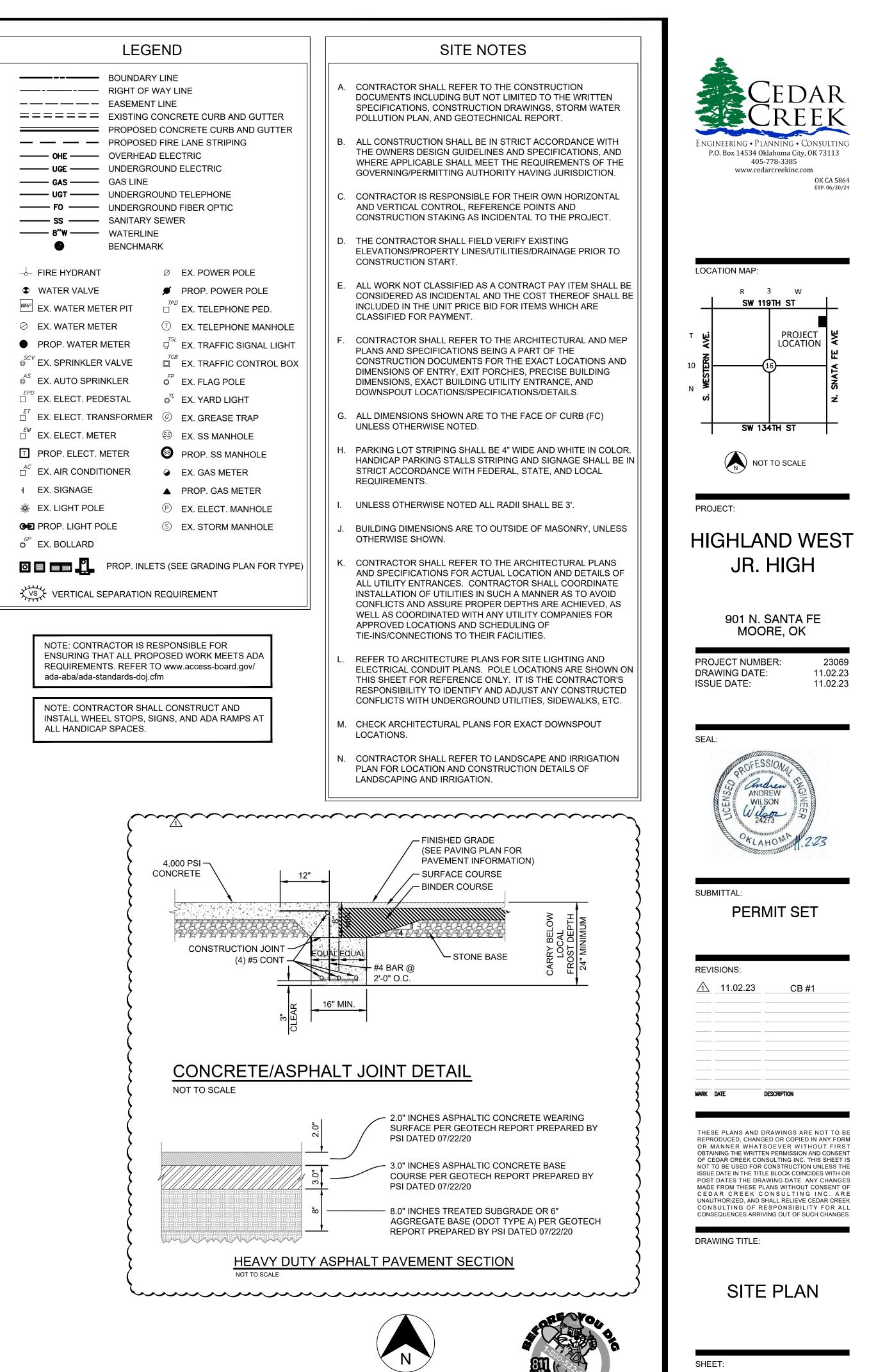
CONSEQUENCES ARRIVING OUT OF SUCH CHANGES.

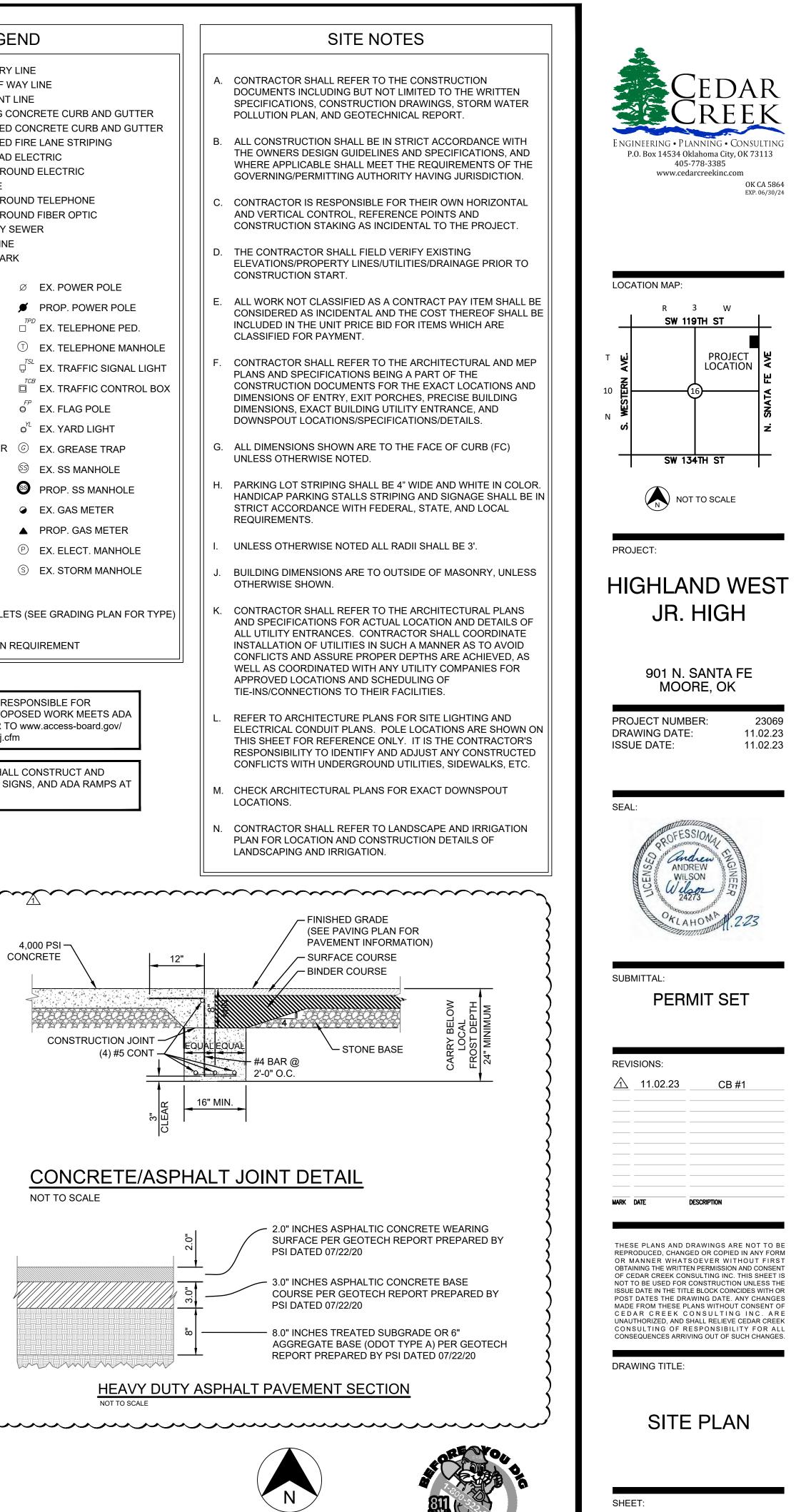
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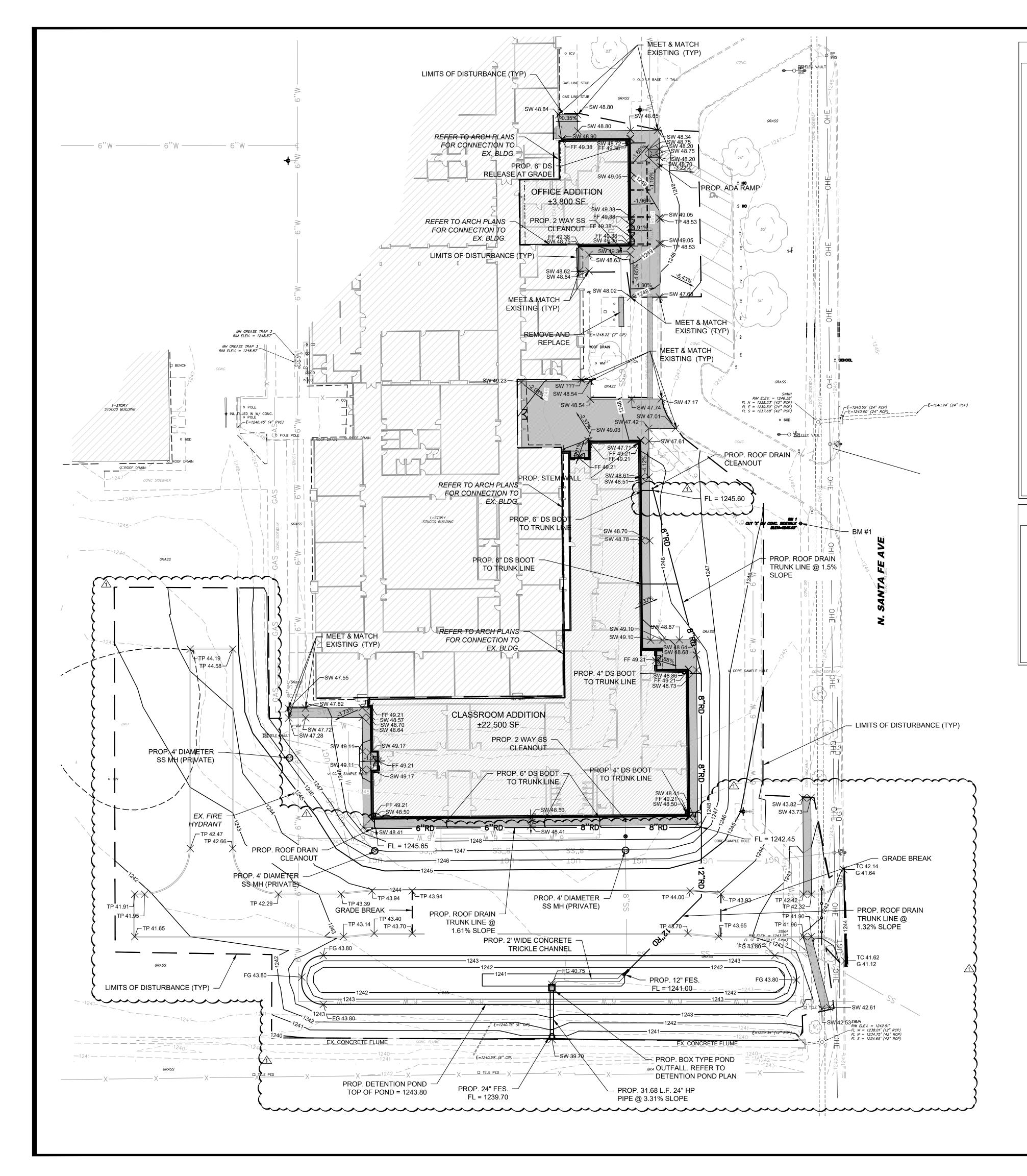
SHEET:







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© ^{AS} EX. AUTO SPRINKLER	0 FP	EX. FLAG
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\square^{ET} EX. ELECT. TRANSFORMER	6	EX. GRE
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₩ EX. LIGHT POLE	P	EX. ELEC
GE PROP. LIGHT POLE	S	EX. STO
e ^{GP} EX. BOLLARD		
	-	

BENCHMARK DAT

BENCHMARK #1 DESC: CONC. SIDEWALK NORTHING: 732420.67 EASTING: 2113951.32 ELEVATION: 1246.12

BENCHMARK #2 DESC: CUT X NORTHING: 732831.70 EASTING: 2113951.35 ELEVATION: 1248.00

VERTICAL DATUM: NAVD 88 OKC GPS MONUMENT

	GRADING NOTES
	A. CONTRACTOR SHALL REFER TO THE SITE SPECIFIC GEOTECHNICAL REPORT FOR EXISTING SOIL CONDITIONS, CONSIDERATIONS, AND RECOMMENDATIONS.
E CURB AND GUTTER TE CURB AND GUTTER IE STRIPING C CTRIC	B. CONTRACTOR SHALL REFER TO THE CONSTRUCTION DOCUMENTS INCLUDING BUT NOT LIMITED TO THE WRITTEN SPECIFICATIONS, CONSTRUCTION DRAWINGS, STORM WATER POLLUTION PLAN, AND GEOTECHNICAL REPORT.
EPHONE R OPTIC	C. CONTRACTOR IS RESPONSIBLE FOR THEIR OWN HORIZONTAL AND VERTICAL CONTROL, REFERENCE POINTS AND CONSTRUCTION STAKING AS INCIDENTAL TO THE PROJECT.
	D. THE CONTRACTOR SHALL FIELD VERIFY EXISTING ELEVATIONS/PROPERTY LINES/UTILITIES/DRAINAGE PRIOR TO CONSTRUCTION START.
POWER POLE	E. ALL SITE EXCAVATION SHALL BE CONSIDERED UNCLASSIFIED EXCAVATION.
TELEPHONE PED. TELEPHONE MANHOLE TRAFFIC SIGNAL LIGHT	F. GENERAL CONTRACTOR TO PROVIDE A UNIT PRICE FOR REMOVAL AND REPLACEMENT OF SOILS ON THIS SITE SHOULD REMOVAL BE REQUIRED.
TRAFFIC CONTROL BOX FLAG POLE YARD LIGHT GREASE TRAP	G. ALL WORK NOT CLASSIFIED AS A CONTRACT PAY ITEM SHALL BE CONSIDERED AS INCIDENTAL AND THE COST THEREOF SHALL BE INCLUDED IN THE UNIT PRICE BID FOR ITEMS WHICH ARE CLASSIFIED FOR PAYMENT.
SS MANHOLE P. SS MANHOLE GAS METER	H. CONTRACTOR SHALL PROVIDE FINAL GRADES THAT DO NOT OBSTRUCT ANY UTILITY ACCESS AND PROVIDE A SMOOTH TRANSITION TO MEET AND MATCH EXISTING GRADES ON ALL SIDES.
P. GAS METER ELECT. MANHOLE STORM MANHOLE	I. ADA ROUTES ARE NOT TO EXCEED 1:20 RUNNING SLOPE AND 2% CROSS SLOPE. HANDICAP PARKING AND ACCESS AISLES SHALL NOT EXCEED 2% IN ANY DIRECTION.
RADING PLAN FOR TYPE)	J. ALL NATURAL GROUND SLOPES SHALL NOT EXCEED 3:1. PAVING SLOPES SHALL NOT EXCEED 8%.
ENT	K. CONTRACTOR SHALL ENSURE THAT ALL NECESSARY EARTH DISTURBING PERMITS HAVE BEEN ACQUIRED AND MEET THE CONDITIONS/REQUIREMENTS SET FORTH IN THE PERMITS PRIOR TO CONSTRUCTION.
TA	L. CONTRACTOR IS REQUIRED TO CALL ONE CALL AS WELL AS THE APPROPRIATE UTILITY COMPANY AT LEAST 48 HOURS BEFORE ANY EXCAVATION/CONSTRUCTION ACTIVITIES TAKE PLACE. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH ARE IN CONFLICT WITH PROPOSED IMPROVEMENTS.
	M. THE CONTRACTOR SHALL GRADE SITE TO ENSURE ALL SURFACE WATER DRAINAGE IS AWAY FROM THE BUILDING AND PROVIDES POSITIVE DRAINAGE SO THAT NO STANDING/PONDING WATER TAKES PLACE ON SITE OR ON ADJACENT PROPERTIES.
	N. ALL CONSTRUCTION SHALL BE IN STRICT ACCORDANCE WITH THE OWNERS DESIGN GUIDELINES AND SPECIFICATIONS, AND WHERE APPLICABLE SHALL MEET THE REQUIREMENTS OF THE GOVERNING/PERMITTING AUTHORITY HAVING JURISDICTION.
	O. THE BUILDING SUBGRADE SHALL BE CONSTRUCTED TO INCLUDE A MINIMUM OF 10 FEET BEYOND THE BUILDING LIMITS AS SHOWN ON THE PLANS, OR AS DIRECTED BY THE OWNER.
	P. REFERENCE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR REQUIRED FLOOR SLAB THICKNESS.
	Q. THE BUILDING PAD SUBGRADE SHALL BE PREPARED IN STRICT ACCORDANCE WITH THE GEOTECHNICAL ENGINEERING STUDY AND THE CIVIL SPECIFICATIONS.
	R. ESTABLISH FINAL SUBGRADE ELEVATIONS TO ALLOW FOR PAVEMENT/SLAB SECTIONS AS INDICATED ON THE PLANS.
	S. IF CONFLICTS EXIST BETWEEN THE GEOTECHNICAL REPORT

Image: Second Second

CB #1

ENGINEERING • PLANNING • CONSULTING P.O. Box 14534 Oklahoma City, OK 73113

405-778-3385

www.cedarcreekinc.com

R 3 W SW 119TH ST

SW 134TH ST

NOT TO SCALE

HIGHLAND WEST

JR. HIGH

901 N. SANTA FE

MOORE, OK

andren

ANDREW

WILSON

Wilson

PERMIT SET

23069

11.02.23

11.02.23

PROJECT NUMBER:

DRAWING DATE:

ISSUE DATE:

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SUBMITTAL:

REVISIONS:

11.02.23

PROJECT:

PROJECT

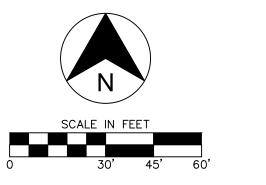
LOCATION MAP:

OK CA 5864 EXP. 06/30/24

OBTAINING THE WRITTEN PERMISSION AND CONSENT OF CEDAR CREEK CONSULTING INC. THIS SHEET IS NOT TO BE USED FOR CONSTRUCTION UNLESS THE ISSUE DATE IN THE TITLE BLOCK COINCIDES WITH OR POST DATES THE DRAWING DATE. ANY CHANGES MADE FROM THESE PLANS WITHOUT CONSENT OF CEDAR CREEK CONSULTING INC. ARE UNAUTHORIZED, AND SHALL RELIEVE CEDAR CREEK CONSULTING OF RESPONSIBILITY FOR ALL CONSEQUENCES ARRIVING OUT OF SUCH CHANGES.

DRAWING TITLE:

GRADING PLAN



VS VERTICAL SEPARATION REQUIREMENT

G - GUTTER

HP - HIGH POINT

LP - LOW POINT

SW - SIDEWALK

TP - TOP OF PAVEMENT



FG - FINAL GRADE

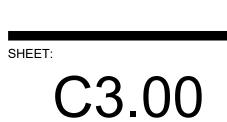
TW - TOP OF WALL

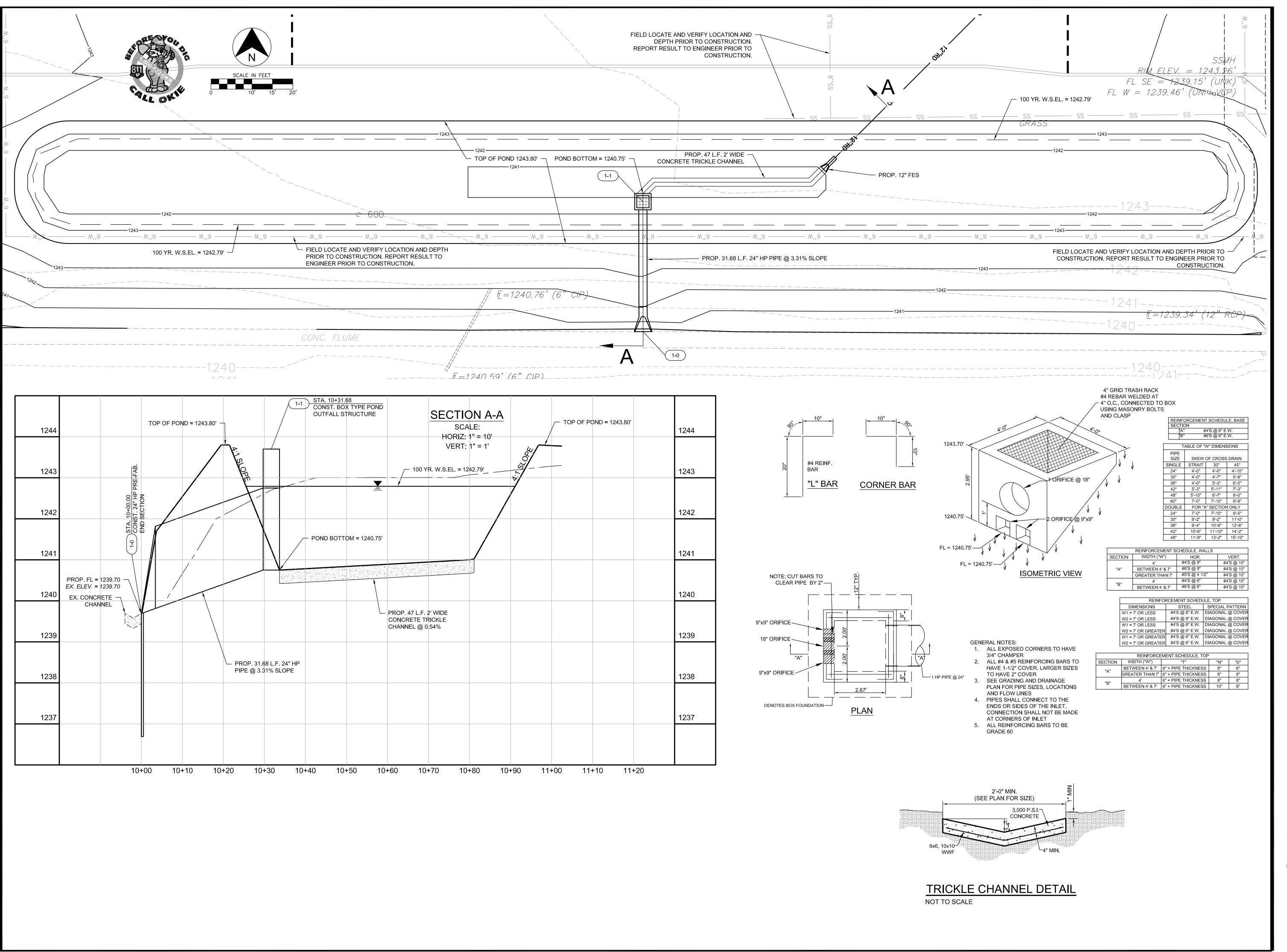
FOOTING

BW - BOTTOM OF WALL

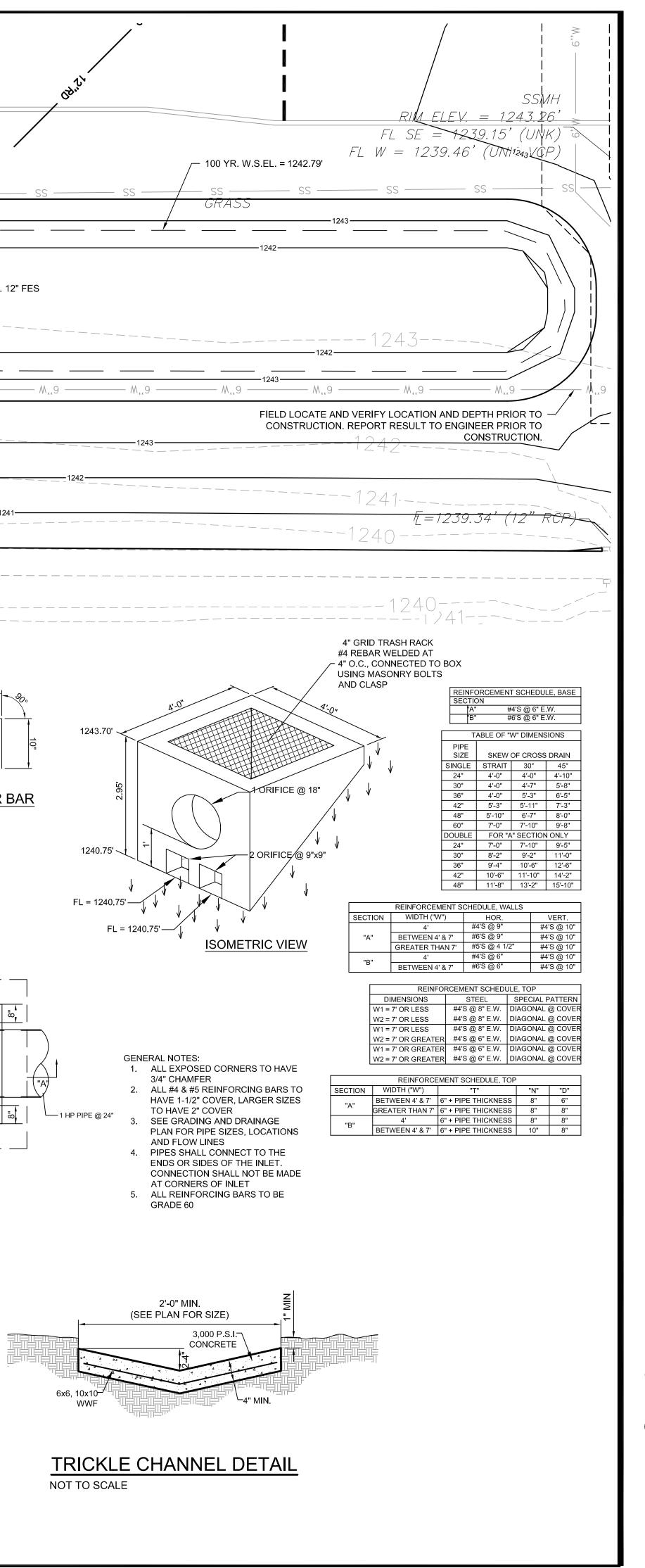
NOTE: BW IS BOTTOM OF

WALL AT GRADE, NOT



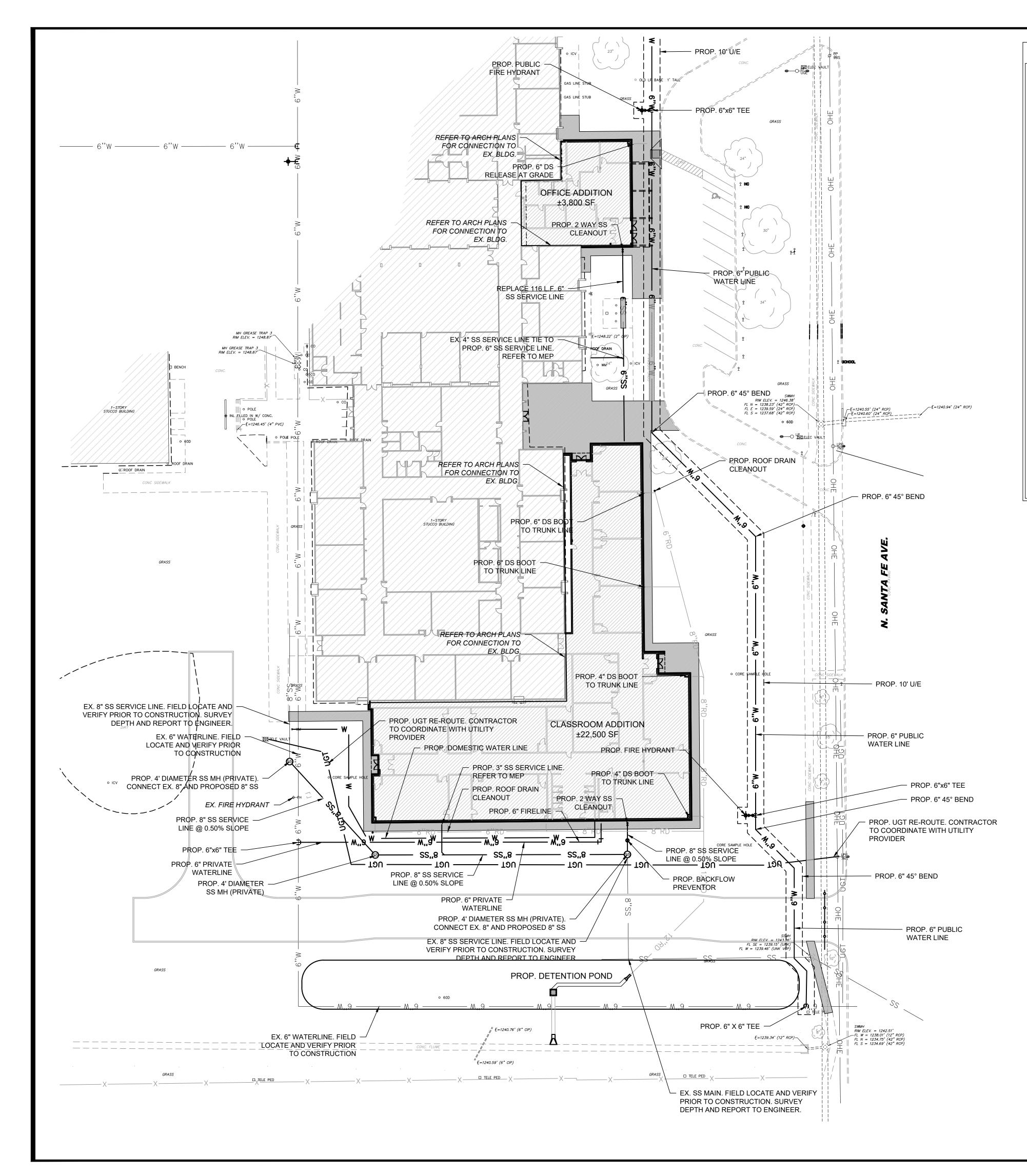


TION A-A SCALE: RIZ: 1" = 10'	/		DND = 1243.80'		1244
ERT: 1" = 1'					
= 1242.79'	5				1243
	7				
					1242
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10+80 10-	+90 11+	-00 11-	-10 11-	+20	



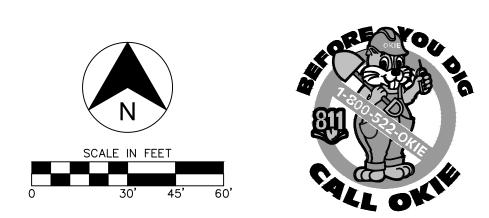
LOC	ATION MAP:		
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		DT TO SCALE	I
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	MOC	ORE, OK	
DR/	DJECT NUM WING DAT JE DATE:		23069 11.02.23 11.02.23
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SUB	MITTAL: PER	MIT SE	ΞT
REV	ISIONS: 11.02.23	СВ	#1
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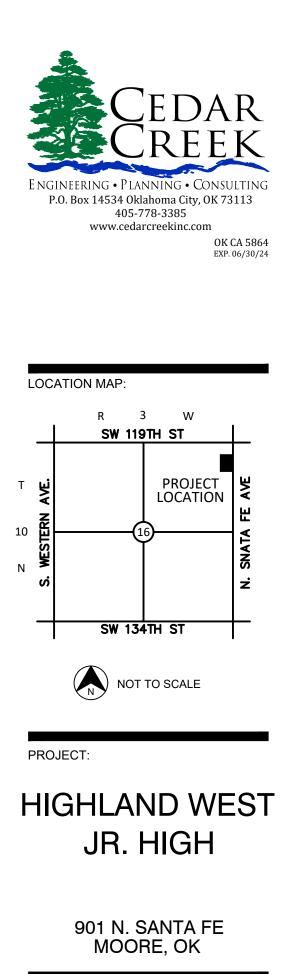


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PROP. WATER METER	TSL	EX. TRAFFIC
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GE PROP. LIGHT POLE	S	EX. STORM
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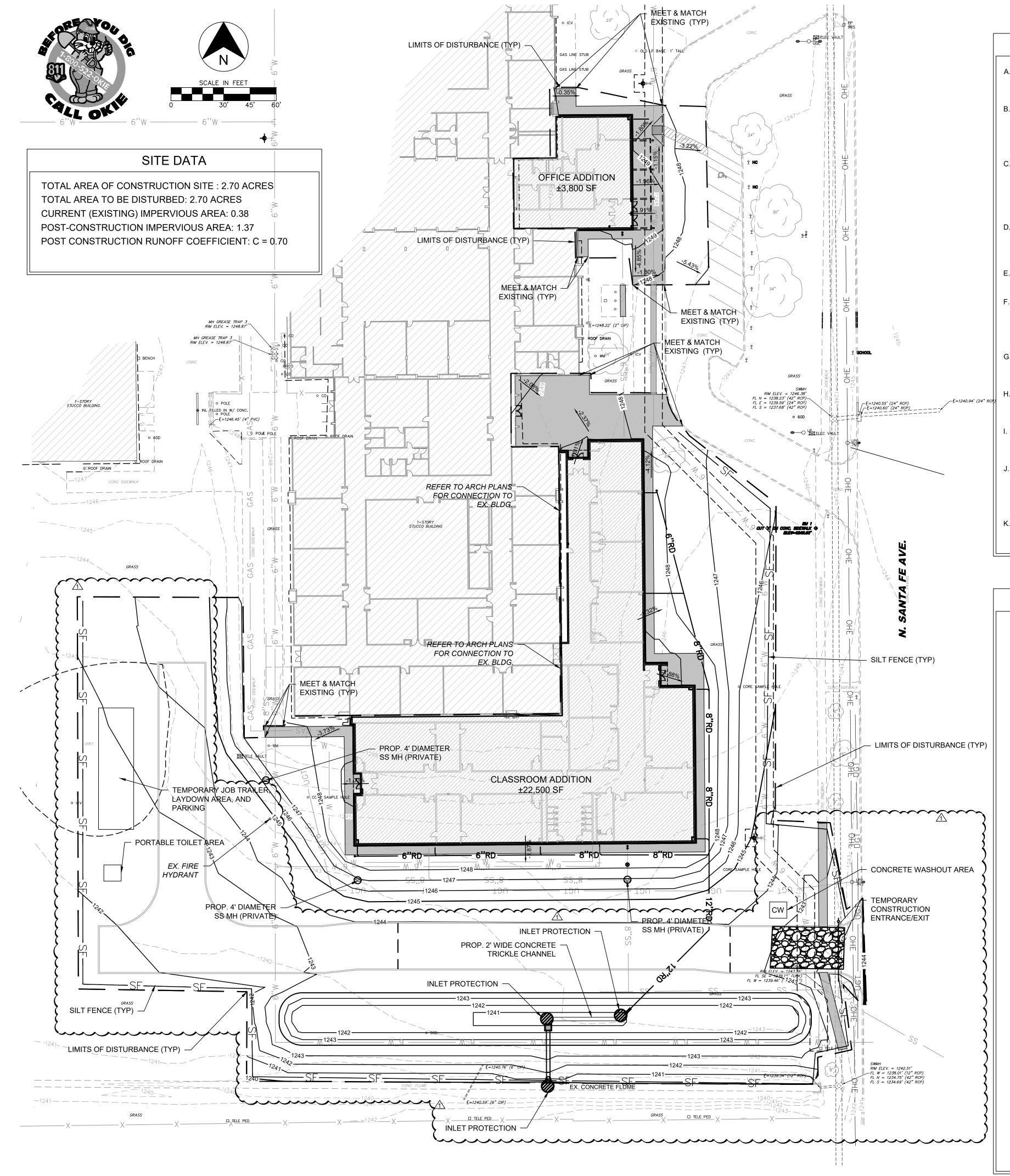
		UTILITY NOTES
	A.	CONTRACTOR SHALL REFER TO THE CONSTRUCTION DOCUMENTS INCLUDING BUT NOT LIMITED TO THE WRITTEN SPECIFICATIONS, CONSTRUCTION DRAWINGS, STORM WATER POLLUTION PLAN, AND GEOTECHNICAL REPORT.
AND GUTTER B AND GUTTER PING	В.	ALL CONSTRUCTION SHALL BE IN STRICT ACCORDANCE WITH THE OWNERS DESIGN GUIDELINES AND SPECIFICATIONS, AND WHERE APPLICABLE SHALL MEET THE REQUIREMENTS OF THE GOVERNING/PERMITTING AUTHORITY HAVING JURISDICTION.
	C.	CONTRACTOR IS RESPONSIBLE FOR THEIR OWN HORIZONTAL AND VERTICAL CONTROL, REFERENCE POINTS AND CONSTRUCTION STAKING AS INCIDENTAL TO THE PROJECT.
	D.	THE CONTRACTOR SHALL FIELD VERIFY EXISTING ELEVATIONS/PROPERTY LINES/UTILITIES/DRAINAGE PRIOR TO CONSTRUCTION START.
POLE (ER POLE	E.	ALL WORK NOT CLASSIFIED AS A CONTRACT PAY ITEM SHALL BE CONSIDERED AS INCIDENTAL AND THE COST THEREOF SHALL BE INCLUDED IN THE UNIT PRICE BID FOR ITEMS WHICH ARE CLASSIFIED FOR PAYMENT.
IONE MANHOLE C SIGNAL LIGHT C CONTROL BOX OLE	F.	CONTRACTOR SHALL REFER TO THE ARCHITECTURAL AND MEP PLANS AND SPECIFICATIONS BEING A PART OF THE CONSTRUCTION DOCUMENTS FOR THE EXACT LOCATIONS AND DIMENSIONS OF ENTRY, EXIT PORCHES, PRECISE BUILDING DIMENSIONS, EXACT BUILDING UTILITY ENTRANCE, AND DOWNSPOUT LOCATIONS/SPECIFICATIONS/DETAILS.
IGHT E TRAP IHOLE IANHOLE	G.	REFER TO ARCHITECTURE PLANS FOR SITE LIGHTING/LIGHT POLE BASES AND ELECTRICAL CONDUIT PLACEMENT AND SPECIFICATIONS. POLE LOCATIONS ARE SHOWN ON THIS SHEET FOR REFERENCE ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO IDENTIFY AND ADJUST ANY CONSTRUCTED CONFLICTS WITH UNDERGROUND UTILITIES, SIDEWALKS, ETC.
TER METER MANHOLE MANHOLE	H.	CONTRACTOR IS REQUIRED TO CALL ONE CALL AS WELL AS THE APPROPRIATE UTILITY COMPANY AT LEAST 48 HOURS BEFORE ANY EXCAVATION/CONSTRUCTION ACTIVITIES TAKE PLACE. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH ARE IN CONFLICT WITH PROPOSED IMPROVEMENTS.
PLAN FOR TYPE)	1.	CONTRACTOR SHALL ENSURE ALL CONSTRUCTED UTILITIES MEET THE MINIMUM SEPARATION AND COVER REQUIREMENTS SET FORTH BY THE PROVIDER, FEDERAL/STATE/LOCAL REGULATIONS, OR SPECIFICATIONS. IN THE EVENT THERE IS A CONFLICT THE MOST STRINGENT SHALL APPLY.
	J.	GENERAL CONTRACTOR TO PROVIDE 2'X2'X6" THICK CONCRETE APRON AT ALL CLEANOUTS, VALVES AND METERS OUTSIDE OF BUILDING.
	К.	GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TAP AND TIE ON FEES REQUIRED, AS WELL AS COST OF UNDERGROUND SERVICE CONNECTIONS TO THE BUILDINGS.
	L.	THRUST BLOCKING SHALL BE PROVIDED AT ALL BENDS, TEES, AND FIRE HYDRANTS.
	M.	DIMENSIONS SHOWN ARE TO CENTERLINE OF PIPE OR FITTING.
	N.	ALL WATER AND SANITARY SEWER LEADS TO BUILDING SHALL END 5' OUTSIDE THE BUILDING LIMITS AS SHOWN ON PLAN AND SHALL BE PROVIDED WITH A TEMPORARY PLUG AT END.
	O.	ALL FIRE HYDRANTS SHALL BE PROVIDED WITH AN APPROVED GATE VALVE A MAXIMUM OF 5'(UNLESS OTHERWISE SPECIFIED BY CITY OFFICIAL) FROM HYDRANT.
	P.	CONTRACTOR SHALL COMPLY COMPLETELY WITH THE LATEST STANDARDS OF OSHA DIRECTIVES OR ANY OTHER AGENCY HAVING JURISDICTION FOR EXCAVATION AND TRENCHING PROCEDURES. THE CONTRACTOR SHALL USE SUPPORT SYSTEMS, SLOPING, BENCHING AND OTHER MEANS OF PROTECTION. THIS IS TO INCLUDE, BUT NOT LIMITED FOR ACCESS AND EGRESS FROM ALL EXCAVATION AND TRENCHING. CONTRACTOR IS RESPONSIBLE FOR COMPLYING WITH PERFORMANCE CRITERIA AS REQUIRED BY OSHA.
	Q.	REFER TO FIRE PROTECTION SHEETS FOR LOCATION AND DETAIL OF FIRE LINE LEAD IN. FIRE LINE SHALL BE STUBBED UP 1' ABOVE FFE IN SPRINKLER ROOM.
	R.	REFER TO PLUMBING SHEETS FOR LOCATION AND DETAILS OF SEWER, DOMESTIC, AND IRRIGATION CONNECTIONS.
	S.	CONTRACTOR SHALL REFER TO IRRIGATION PLANS FOR ACTUAL LOCATION, SIZE, LENGTH AND DEPTH. TEMPORARILY PLUG BOTH ENDS. IRRIGATION CONTRACTOR WILL REMOVE TEMPORARY PLUGS, INSTALL LINES AND PROPERLY SEAL BOTH ENDS.
	т.	THE FIRE DEPARTMENT CONNECTION (FDC) SHALL BE LOCATED ON THE STREET SIDE OF ANY STRUCTURE. THE FDC SHALL BE LOCATED AND ARRANGED SO THAT THE HOSE LINES CAN BE READILY ATTACHED TO THE INLETS WITHOUT INTERFERENCE FROM OBJECTS.
	U.	IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE THE EXTENSIONS OF ALL UTILITY SERVICE LINES TO THE MAIN UTILITY LINES.
	V.	ALL CONDUIT SHALL BE SCHEDULE 40 PVC, UNLESS OTHERWISE NOTED.
	w.	CONTRACTOR SHALL REFER TO LANDSCAPE AND IRRIGATION PLAN FOR LOCATION AND CONSTRUCTION DETAILS OF



LANDSCAPING AND IRRIGATION.



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	MITTAL: PE /ISIONS: 	RMIT S	B #1
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CON CON	ISULTING O SEQUENCES A	F RESPONSIE RRIVING OUT OI	LAN
SHE	C	4.0	0



A. SEDIMENT BASINS ARE ATTRACTIVE TO CHILDREN AND CAN BE VERY DANGEROUS. IN ALL CASES, LOCAL ORDINANCES AND REGULATIONS

REGARDING HEALTH AND SAFETY MUST BE ADHERED TO.

- B. ALL CONTRACTORS AND SUBCONTRACTORS INVOLVED WITH STORM WATER POLLUTION PREVENTION SHALL OBTAIN A COPY OF THE STORM WATER POLLUTION PREVENTION PLAN AND THE STATE OF OKLAHOMA NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM GENERAL PERMIT (NPDES PERMIT) AND BECOME FAMILIAR WITH THEIR CONTENTS.
- C. ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHALL BE DISPOSED OF WITHIN 30 DAYS AFTER FINAL STABILIZATION. FINAL STABILIZATION HAS OCCURRED WHEN ALL SOIL DISTURBING ACTIVITIES ARE COMPLETED AND A UNIFORM PERENNIAL VEGETATIVE COVER WITH A DENSITY OF 70% OF THE COVER FOR UNPAVED AREAS AND AREAS NOT COVERED BY PERMANENT STRUCTURES HAS BEEN EMPLOYED.
- D. BEST MANAGEMENT PRACTICES (BMP'S) AND CONTROLS SHALL CONFORM TO FEDERAL, STATE, OR LOCAL REQUIREMENTS OR MANUAL OF PRACTICE, AS APPLICABLE CONTRACTOR SHALL IMPLEMENT ADDITIONAL CONTROLS AS DIRECTED BY PERMITTING AGENCY OR OWNER.
- . CONTRACTOR SHALL MINIMIZE CLEARING TO THE MAXIMUM EXTENT PRACTICAL OR AS REQUIRED BY THE GENERAL PERMIT.
- GENERAL CONTRACTOR SHALL DENOTE ON PLAN THE TEMPORARY PARKING AND STORAGE AREA WHICH SHALL ALSO BE USED AS THE EQUIPMENT MAINTENANCE AND CLEANING AREA, EMPLOYEE PARKING AREA, AND AREA FOR LOCATING PORTABLE FACILITIES, OFFICE TRAILERS, AND TOILET FACILITIES.
- G. ALL WASH WATER (CONCRETE TRUCKS, VEHICLE CLEANING, EQUIPMENT CLEANING, ETC.) SHALL BE DETAINED AND PROPERLY TREATED OR DISPOSED.
- H. SUFFICIENT OIL AND GREASE ABSORBING MATERIALS AND FLOTATION BOOMS SHALL BE MAINTAINED ON SITE OR READILY AVAILABLE TO CONTAIN AND CLEAN-UP FUEL OR CHEMICAL SPILLS AND LEAKS.
- DUST ON THE SITE SHALL BE CONTROLLED. THE USE OF MOTOR OILS AND OTHER PETROLEUM BASED OR TOXIC LIQUIDS FOR DUST SUPPRESSION OPERATIONS IS PROHIBITED.
- RUBBISH, TRASH, GARBAGE, LITTER, OR OTHER SUCH MATERIALS SHALL BE DEPOSITED INTO SEALED CONTAINERS. MATERIALS SHALL BE PREVENTED FROM LEAVING THE PREMISES THROUGH THE ACTION OF WIND OR STORMWATER DISCHARGE INTO DRAINAGE DITCHES OR WATERS OF THE STATE.
- K. ALL STORM WATER POLLUTION PREVENTION MEASURES PRESENTED ON THIS PLAN, AND IN THE STORM WATER POLLUTION PREVENTION PLAN, SHALL BE INITIATED AS SOON AS PRACTICABLE.

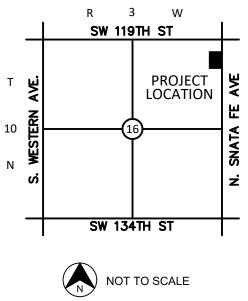
LEGEND	SEQUENCE OF CONSTRUCTION
BOUNDARY LINE RIGHT OF WAY LINE RIGHT OF WAY LINE EASEMENT LINE EASEMENT LINE PROPOSED CONCRETE CURB AND GUTTER GAS GAS LINE UNDERGROUND FIBER OPTIC SS SANITARY SEWER BENCHMARK PROP. POWER POLE WATER VALVE PROP. POWER POLE WATER VALVE PROP. POWER POLE WATER VALVE PROP. POWER POLE PROP. WATER METER EX. TRAFFIC SIGNAL LIGHT SEX. SPRINKLER VALVE SEX. FLAG POLE PROP. WATER METER SEX. FLAG POLE PROP. ELECT. METER PROP. SS MANHOLE	 SEQUENCE OF CONSTRUCTION PHASE 1 A PRE-CONSTRUCTION MEETING SHALL BE HELD BY THE GENERAL CONTRACTOR'S MANAGER, AND THE OPERATOR'S ENGINEER PRIOR TO LAND DISTURBING ACTIVITIES. PREPARE AND PULL ALL NECESSARY PERMITS. CONSTRUCT TEMPORARY CONSTRUCTION EXITS AT LOCATIONS SHOWN ON THE SWPPP PLANS AND PREPARE TEMPORARY PARKING AND STORAGE AREA. UPON IMPLEMENTATION AND INSTALLATION OF THE FOLLOWING AREAS: TRALER, PARKING, LAY DOWN, PORTA-POTTY, WELL WASH, CONCRETE WASH-OUT, MASONS AREA, FUEL AND MATERIAL STORAGE CONTAINERS, SOLID WASTE CONTAINERS, ETC., DENOTE THEM ON THE SITE MAPS INMEDIATELY AND NOTE ANY CHANGE IN THE LOCATIONS AS THEY OCCUR THROUGHOUT THE CONSTRUCTION PROCESS. CONSTRUCT THE SILT FENCES ON THE SITE. HALT ALL ACTIVITIES AND CONTACT THE CIVIL ENGINEERING CONSULTANT TO PERFORM INSPECTION AND CERTIFICATION OF BMPS. GENERAL CONTRACTOR SHALL SCHEDULE AND CONDUCT STORMWATER PRE-CONSTRUCTION MEETING WITH ENGINEER AND ALL GROUND-DISTURBING CONTRACTORS BEFORE PROCEEDING WITH CONSTRUCTION. INSTALL PUBLIC WATER, SEWER AND BOX CULVERT DEMO, CLEAR AND GRUB THE SITE. BEGIN GRADING THE SITE. START CONSTRUCTION OF BUILDING PAD AND STRUCTURES. DISTURBED AREAS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS CEASED FOR MORE THAN 14 DAYS SHALL BE TEMPORARILY SEEDED AND GUTTERS. INSTALL NUET PROTECTION DEVICES. INSTALL RIP RAP AROUND OUTLET STRUCTURES. INSTALL RIP RAP AROUND OUTLET STRUCTURES. INSTALL RIP RAP AROUND OUTLET STRUCTURES. INSTALL BASE MATERIAL AS REQUIRED FOR PAVEMENT. PAVE LOT. REMOVE TEMPORARY CONSTRUCTION EXITS ONLY PRIOR TO PAVEMENT CONSTRUCTION IN THESE AREAS. (THESE AREAS TO BE PAVED LAST) DISTUBBED AREAS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS CEASED FOR MORE THAN 14 DAYS SHALL BE TEMPORARILY PRIOR TO PAVEMENT CONSTRUCTION IN THESE AREAS. (THESE AREAS TO BE P
CW CONCRETE WASHOUT AREA	IF SITE IS STABILIZED) 11. REMOVE INLET PROTECTIONS AROUND INLETS AND MANHOLES NO MORE THAN 48 HOURS PRIOR TO PLACING STABILIZED BASE COURSE.

EROSION CONTROL NOTES

L. DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS STOPPED FOR AT LEAST 14 DAYS, SHALL BE TEMPORARILY SEEDED. THESE AREAS SHALL BE SEEDED NO LATER THAN 14 DAYS FROM THE LAST CONSTRUCTION ACTIVITY OCCURRING IN THESE AREAS.

- M. DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS PERMANENTLY STOPPED SHALL BE PERMANENTLY SEEDED. THESE AREAS SHALL BE SEEDED NO LATER THAN 14 DAYS AFTER THE LAST CONSTRUCTION ACTIVITY OCCURRING IN THESE AREAS. REFER TO THE GRADING PLAN AND/OR LANDSCAPE PLAN.
- N. IF THE ACTION OF VEHICLES TRAVELING OVER THE GRAVEL CONSTRUCTION ENTRANCES IS NOT SUFFICIENT TO REMOVE THE MAJORITY OF DIRT OR MUD, THEN THE TIRES MUST BE WASHED BEFORE THE VEHICLES ENTER A PUBLIC ROAD. IF WASHING IS USED, PROVISIONS MUST BE MADE TO INTERCEPT THE WASH WATER AND TRAP THE SEDIMENT BEFORE IT IS CARRIED OFF THE SITE.
- O. ALL MATERIALS SPILLED, DROPPED, WASHED, OR TRACKED FROM VEHICLES ONTO ROADWAYS OR INTO STORM DRAINS MUST BE REMOVED IMMEDIATELY.
- P. CONTRACTORS OR SUBCONTRACTORS WILL BE RESPONSIBLE FOR REMOVING SEDIMENT IN THE DETENTION POND AND ANY SEDIMENT THAT MAY HAVE COLLECTED IN THE STORM SEWER DRAINAGE SYSTEMS IN CONJUNCTION WITH THE STABILIZATION OF THE SITE.
- Q. ON-SITE & OFFSITE SOIL STOCKPILE AND BORROW AREAS SHALL BE PROTECTED FROM EROSION AND SEDIMENTATION THROUGH IMPLEMENTATION OF BEST MANAGEMENT PRACTICES. STOCKPILE AND BORROW AREA LOCATIONS SHALL BE NOTED ON THE SITE PLAN AND PERMITTED IN ACCORDANCE WITH GENERAL PERMIT REQUIREMENTS.
- R. SLOPES SHALL BE LEFT IN A ROUGHENED CONDITION DURING THE GRADING PHASE TO REDUCE RUNOFF VELOCITIES AND EROSION.
- S. DUE TO THE GRADE CHANGES DURING THE DEVELOPMENT OF THE PROJECT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADJUSTING THE EROSION CONTROL MEASURES (SILT FENCES, STRAW BALES, ETC.) TO PREVENT EROSION.
- T. ALL CONSTRUCTION SHALL BE STABILIZED AT THE END OF EACH WORKING DAY, THIS INCLUDES BACKFILLING OF TRENCHES FOR UTILITY CONSTRUCTION AND PLACEMENT OF GRAVEL OR BITUMINOUS PAVING FOR ROAD CONSTRUCTION.
- U. A 3' STRIP OF SOD SHALL BE PLACED ALONG THE EDGE OF ALL PAVING TO ACT AS A SEDIMENT BUFFER AND AID IN THE ESTABLISHMENT OF VEGETATION.





PROJECT:

HIGHLAND WEST JR. HIGH

901 N. SANTA MOORE, OK	FE
PROJECT NUMBER:	23069
DRAWING DATE:	11.02.23

11.02.23

ISSUE DATE:



SUBMITTAL:

PERMIT SET

REVISIONS: 11.02.23

CB #1

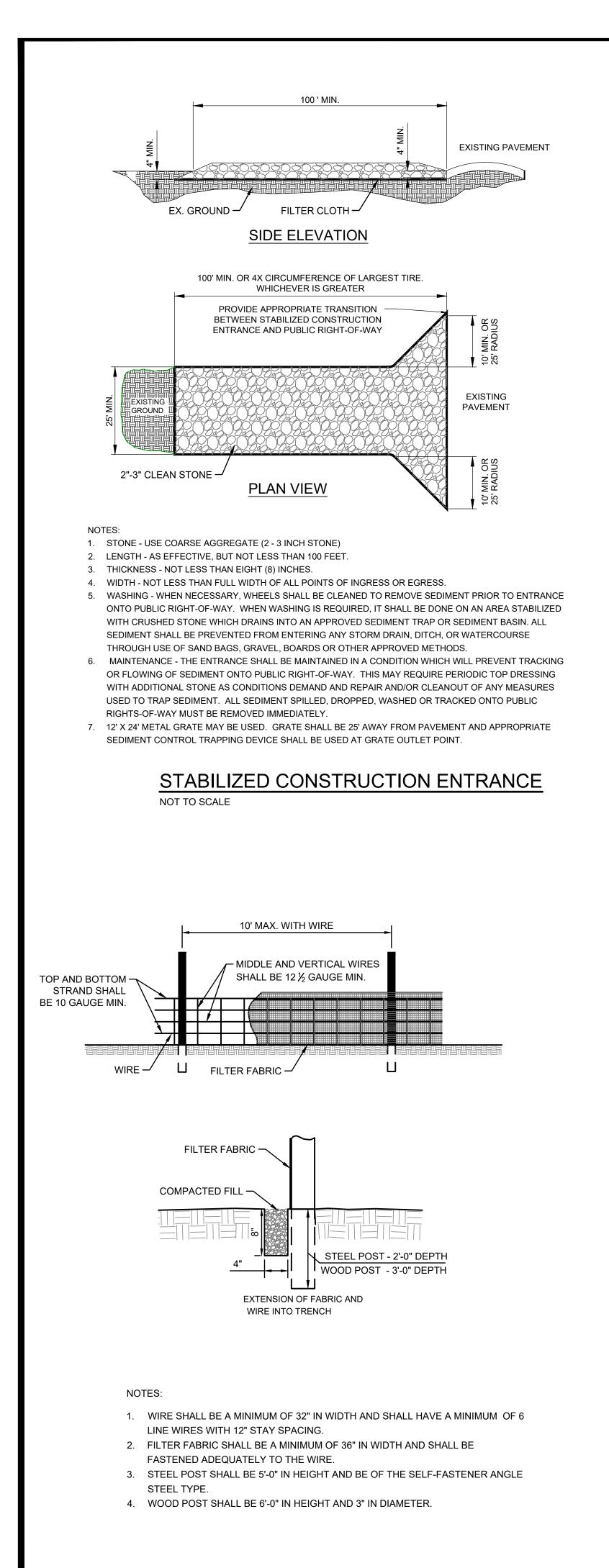
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CONSULTING OF RESPONSIBILITY FOR ALL CONSEQUENCES ARRIVING OUT OF SUCH CHANGES.

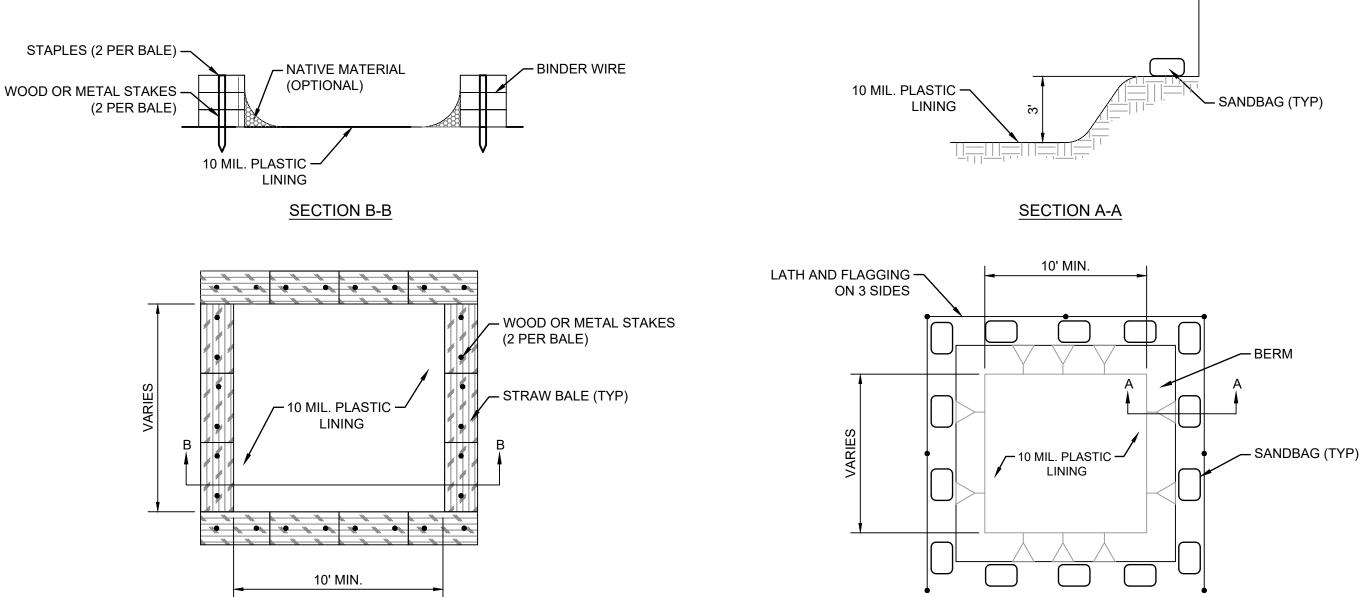
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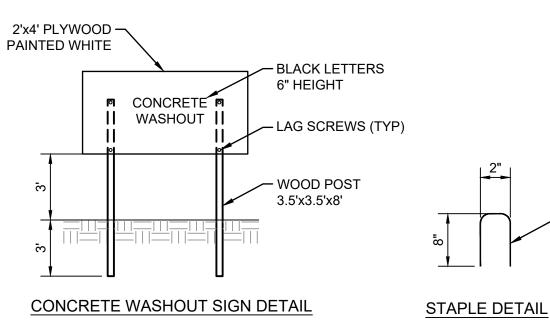
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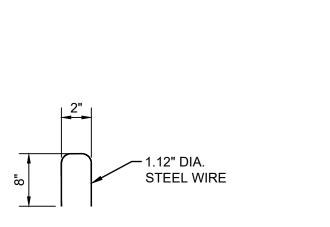


SILT FENCE DETAIL NOT TO SCALE



TYPE 'ABOVE GRADE' WITH STRAW BALES







NOTES:

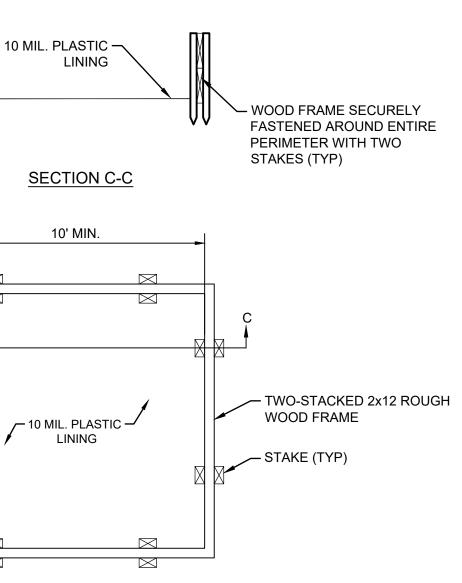
- 1. ACTUAL LAYOUT TO BE DETERMINED IN THE FIELD. 2. A CONCRETE WASHOUT SIGN SHALL BE INSTALLED WITHIN 30' OF THE
- TEMPORARY CONCRETE WASHOUT FACILITY
- 3. MATERIALS USED TO CONSTRUCT TEMPORARY CONCRETE WASHOUT FACILITIES SHALL BE REMOVED FROM THE SITE OF THE WORK AND DISPOSED OF OR RECYCLED.
- 4. HOLES, DEPRESSIONS OR OTHER GROUND DISTURBANCE CAUSE BY THE REMOVAL OF THE TEMPORARY CONCRETE WASHOUT FACILITIES SHALL BE BACKFILLED, REPAIRED AND STABILIZED TO PREVENT EROSION.

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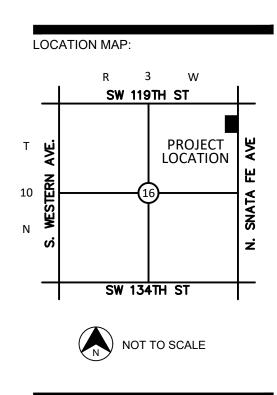
CONCRETE WASHOUT DETAIL NOT TO SCALE

TYPE 'BELOW GRADE'



TYPE 'ABOVE GRADE' WITH WOOD PLANKS





PROJECT:

HIGHLAND WEST JR. HIGH

901 N. SANTA MOORE, OI	
ROJECT NUMBER:	23069
RAWING DATE:	11.02.23
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SFAL





SUBMITTAL:

PERMIT SET

REVISIONS: 11.02.23

CB #1

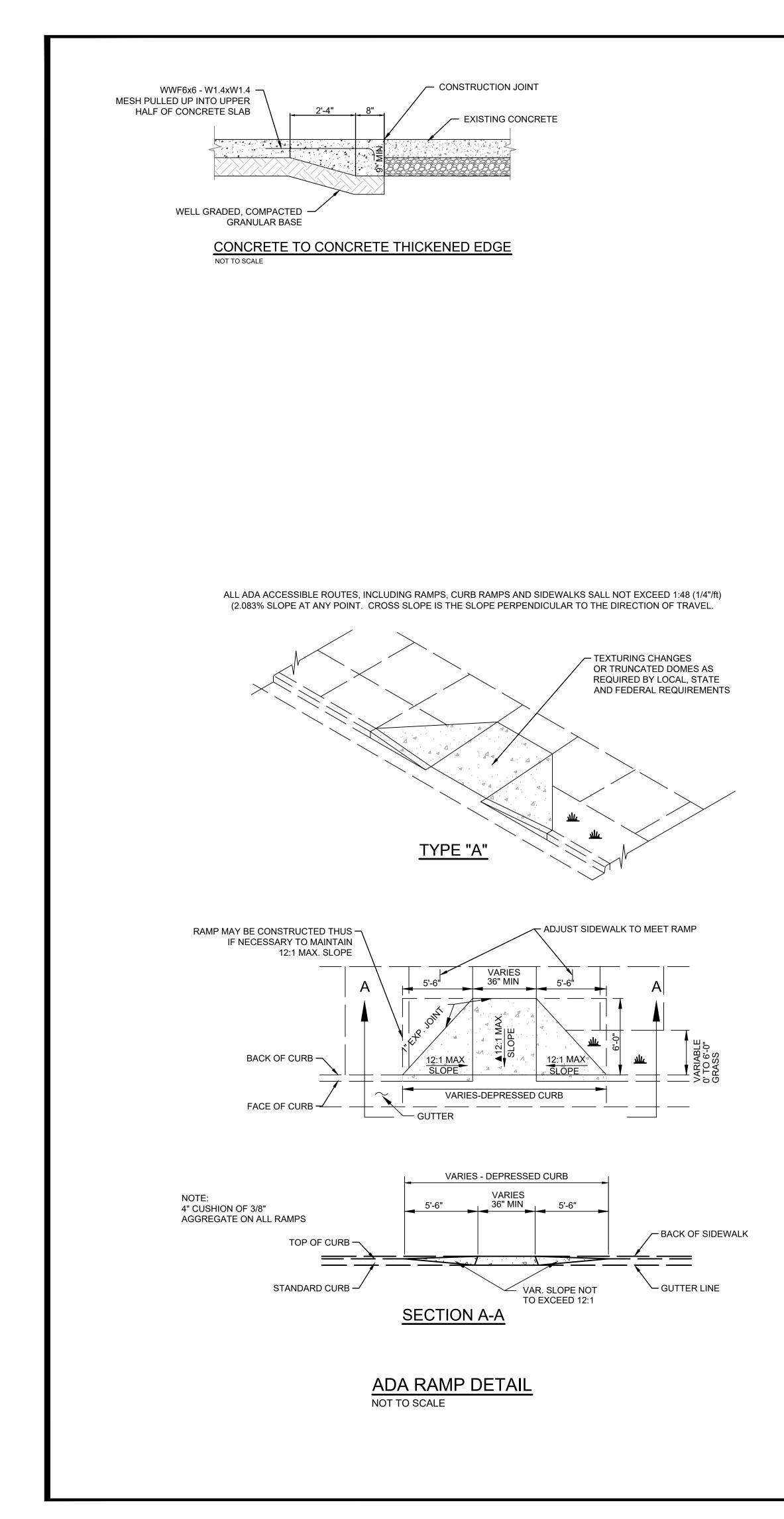
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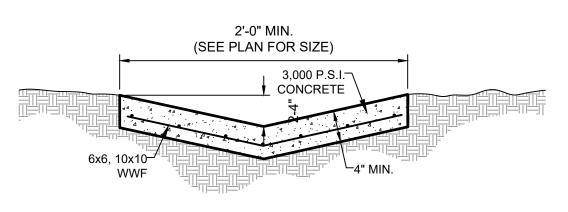
CONSEQUENCES ARRIVING OUT OF SUCH CHANGES.

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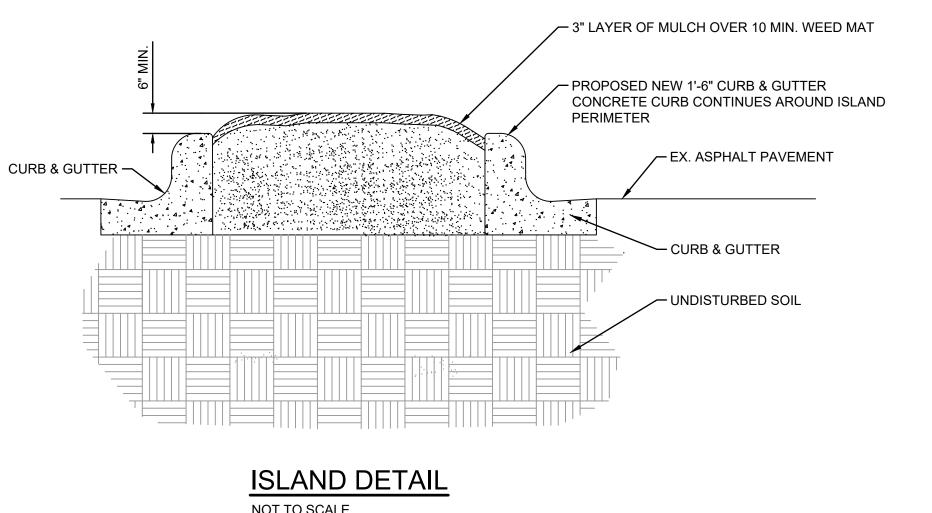
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TRICKLE CHANNEL DETAIL

NOT TO SCALE

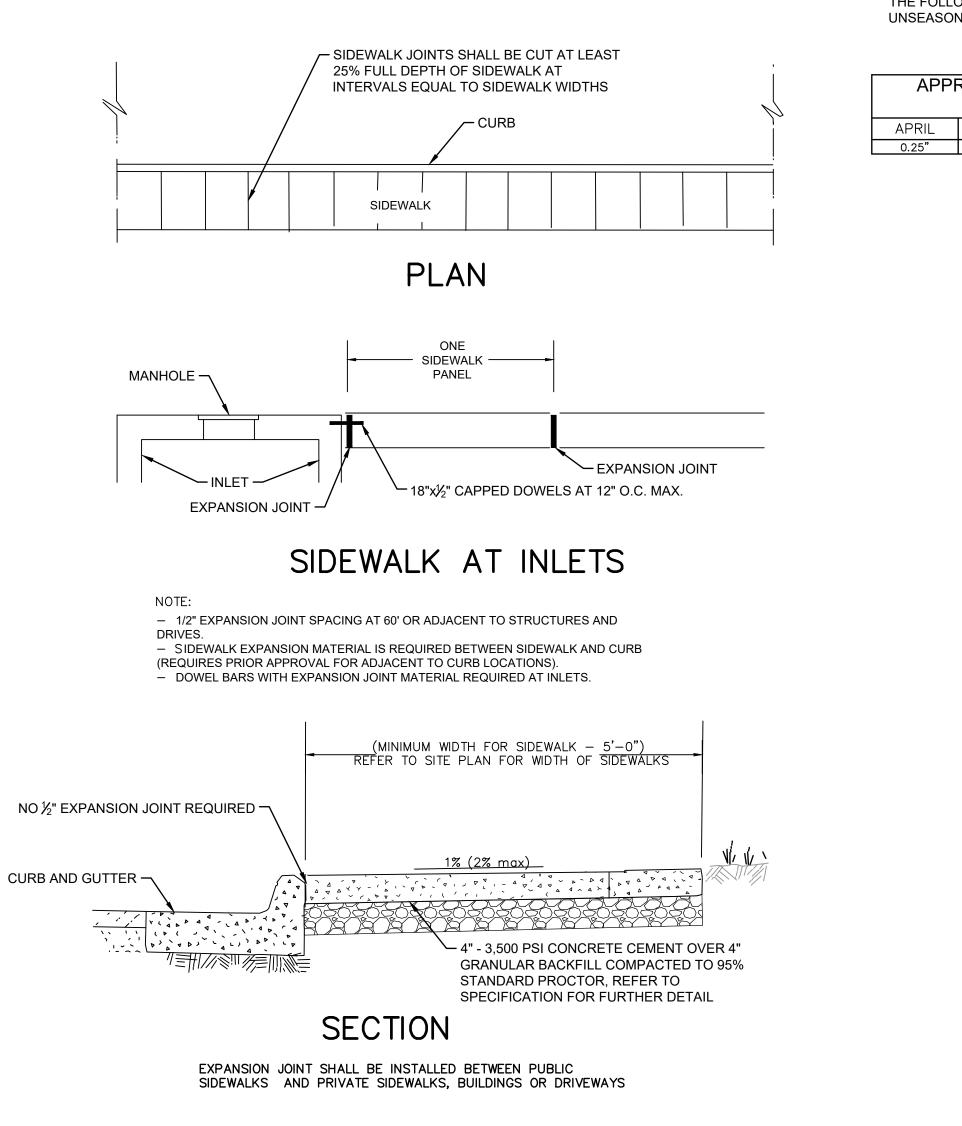


SOD AREA: RAISE SOD 1/4" OVER -WALKS/CURBS TO ALLOW FOR SETTLING

SHREDDED TOPSOIL -

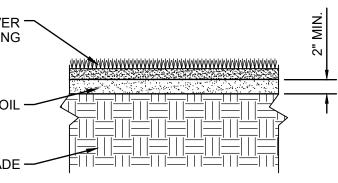
EX. SUBGRADE -

NOTE:



SIDEWALK DETAIL NOT TO SCALE

NOT TO SCALE



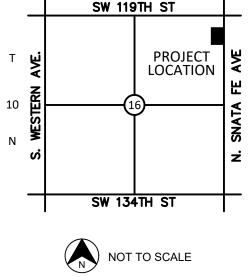
REMOVE WEEDS AND DEBRIS, RAKE ALL SURFACE AREAS SMOOTH PRIOR TO LAYING SOD OR SEEDING. SLOPE TO DRAIN AWAY FROM BUILDINGS. ADHERE TO THE FOLLOWING SUPPLEMENTAL WATERING SCHEDULE (ADJUST AS NEEDED FOR UNSEASONAL RAINFALL CONDITIONS):

APPROXIMATE SUPPLEMENTAL WATER FOR AN AVERAGE						
TRADITIONAL LAWN (INCHES PER WEEK)						
PRIL	MAY	JUNE	JULY	AUGUST	SEPTEMBER	OCTOBER
0.25"	0.75"	1.25"	1.25"	1.0"	0.75"	0.5"

SOD PLANTING DETAIL

NOT TO SCALE





PROJECT:

HIGHLAND WEST JR. HIGH

901 N. SANTA FE MOORE, OK

PROJECT NUMBER:	
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23069



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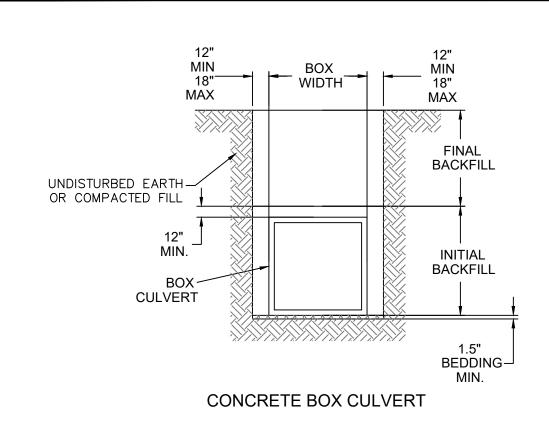
UNAUTHORIZED, AND SHALL RELIEVE CEDAR CREEK CONSULTING OF RESPONSIBILITY FOR ALL CONSEQUENCES ARRIVING OUT OF SUCH CHANGES.

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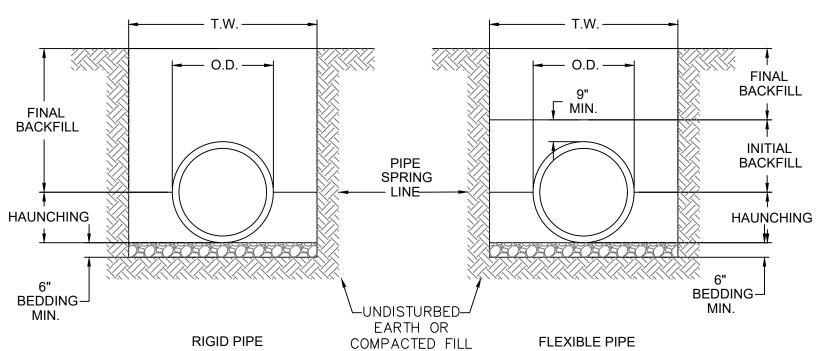


SHEET:





- 1. BEDDING SHALL BE DUMPED CLASS I-A WORKED BY HAND, OR CLASS I-B COMPACTED TO 85% STANDARD PROCTOR. LOCAL CODE PERMITTING WITH GEOTECHNICAL ENGINEER AND OWNER APPROVAL, NATIVE SOIL MAY BE USED FOR BEDDING PROVIDED IT MEETS THE EMBEDMENT AND BACKFILL MATERIALS IN TABLE 1 EXCLUDING CLASS IV-A.
- 2. HAUNCHING SHALL BE WORKED AROUND THE PIPE BY HAND TO ELIMINATE VOIDS AND SHALL BE CLASS I-A, OR CLASS I-B OR CLASS II COMPACTED TO 95% STANDARD PROCTOR. PEA GRAVEL SHALL NOT BE USED AS A HAUNCHING MATERIAL. CLASS III MATERIAL SHALL BE ALLOWED FOR RIGID PIPE COMPACTED AT 95% STANDARD PROCTOR.
- 3. INITIAL BACKFILL SHALL BE CLASS I-A WORKED BY HAND, OR CLASS I-B OR CLASS II COMPACTED TO 90% STANDARD PROCTOR, OR CLASS III COMPACTED 95% STANDARD PROCTOR. CLASS I & II MATERIAL SHALL BE USED FOR FLEXIBLE PIPE WHEN FILL HEIGHTS EXCEED 8'.

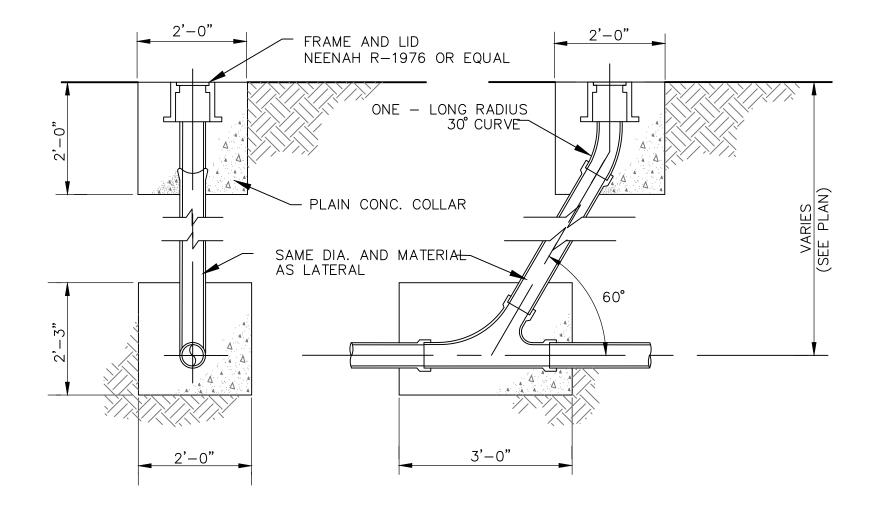


RIGID PIPE (RCP) OR (CMP & SPIRAL RIB48"Ø)

- 4. FINAL BACKFILL SHALL BE CLASS I-A WORKED BY HAND, OR CLASS I-B OR CLASS II COMPACTED TO 90% STANDARD PROCTOR, OR CLASS III COMPACTED TO 95% STANDARD PROCTOR.
- 5. FINAL BACKFILL NOT UNDER PAVED AREAS CAN BE CLASS IV-A COMPACTED TO 95% STANDARD PROCTOR.
- 6. ALL MATERIALS ARE CLASSIFIED IN ACCORDANCE WITH ASTM D 2321. (SEE TABLE 1)
- 7. ALL MATERIALS SHALL BE INSTALLED IN MAXIMUM 8" LOOSE LIFTS IN ACCORDANCE WITH ASTM D 698. CLASS III AND IV-A MATERIALS SHALL BE COMPACTED NEAR OPTIMUM MOISTURE CONTENT.
- 8. FILL SALVAGED FROM EXCAVATION SHALL BE FREE OF DEBRIS, ORGANICS AND ROCKS LARGER THAN 3".
- 9. ALL TRENCH EXCAVATIONS SHALL BE SLOPED, SHORED, SHEETED, BRACED, OR OTHERWISE SUPPORTED IN COMPLIANCE WITH OSHA REGULATIONS AND LOCAL ORDINANCES.

ASTM D 2321 MATERIAL	ASTM D 2487 USCS	MATERIAL TYPE		% PASSING		ATTERBERG LIMITS	
CLASS	SOIL GROUP		1 1/2 IN.	NO. 4	NO. 200	LL	PI
IA	NONE	MANUFACTURED OPEN GRADED AGGREGATES	100%	<u><</u> 10%	<5%	NON	I PLASTIC
IB	NONE	MANUFACTURED DENSE GRADED AGGREGATES	100%	<u>≤</u> 50%	<5%	NON	I PLASTIC
	GW			<50% OF "COARSE			
	GP	COARSE-GRAINED SOILS, CLEAN	100%	FRACTION"	<5%	NON PLASTIC	
	SW			>50% OF "COARSE			
	SP			FRACTION"			
	GM		100%		12% TO		<4 OR <"A" LINE
	GC						<7 OR >"A" LINE
	SM	COARSE-GRAINED SOILS W/ FINES			50%		>4 OR <"A" LINE
	SC						>7 OR >"A" LINE
IV-A	ML		100%	100%	>50%	<50	<4 OR <"A" LINE
	CL	FINE-GRAINED SOILS	10070	10070			>7 OR >"A" LINE

TABLE 1: CLASSES OF EMBEDMENT AND BACKFILL MATERIALS



TYPICAL CLEANOUT DETAIL NOT TO SCALE

10. DESIGN ENGINEER SHALL DESIGNATE ON THE PLANS WHERE WATERTIGHT JOINTS ARE TO BE REQUIRED.

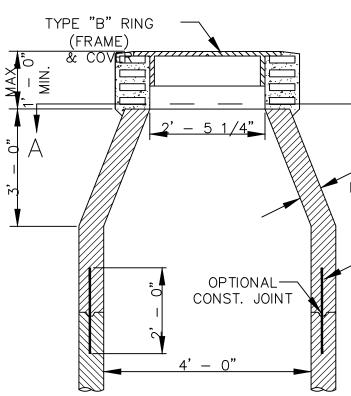
(HDPE) OR (PVC) OR (CMP 48"Ø)

- 11. REPLACE WET OR UNSUITABLE SOIL AS NECESSARY TO PROVIDE A SUITABLE BASE, AS DIRECTED BY GEOTECHNICAL ENGINEER OR OWNER.
- 12. WHERE GROUND WATER IS PRESENT CLASS I-A MATERIAL SHALL BE WRAPPED WITH A NON-WOVEN GEO-TEXTILE, EXCLUDING BEDDING MATERIAL BETWEEN 4" & 6" THICK.
- 13. CONTRACTOR SHALL REFER TO GEOTECHNICAL REPORT FOR SOIL TYPE AND CLASSIFICATIONS FOR THIS PROJECT.
- 14. CONTRACTOR SHALL REFER TO THE LATEST VERSION OF ASTM STANDARDS PRIOR TO CONSTRUCTION.

CHAMFER CORNERS AT ASPHALT ONLY -SQUARE CORNERS AT CONCRETE

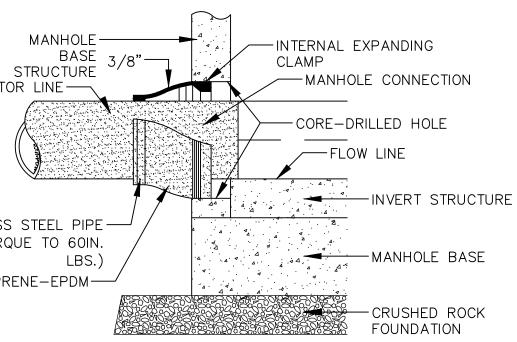
BRASS RECESSED PLUG

TYPICAL CLEANOUT MANHOLE DETAIL NOT TO SCALE



NOTE: STEEL

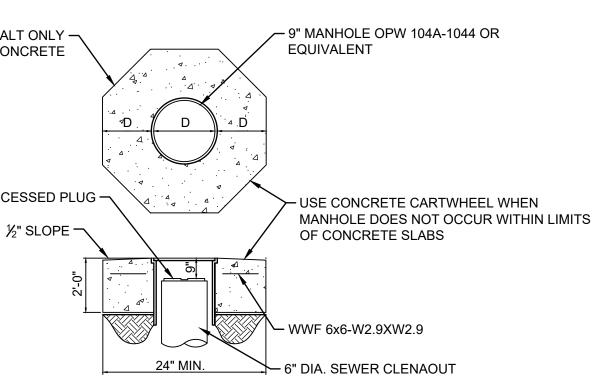




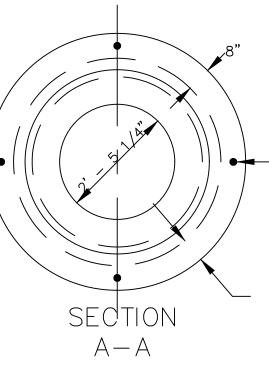
STAINLESS STEEL PIPE-CLAMP (TORQUE TO 60IN.

NEOPRENE-EPDM-

TRENCH AND BEDDING DETAILS NOT TO SCALE



-CONE & WALL MINIMUM THICKNESS 4 – #4 BARS 2' – 0" LONG



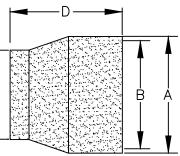
• 4 – #4 PREFORMED BARS X 2' EQUALLY SPACED

> SEE BASE SECTION DETAILS

CAST-IN-PLACE NON-REINFORCED MANHOLE-PIPE CONNECTION

1. ALL PIPE SHALL BE STAINLESS

2. NEOPRENE--EPDM BLENDED COMPOUND BOOT SHALL MEET ASTM C-923



SUGGESTED PIPE O.D.	HOLE & BOOT DIAMETER DIMENSIONS				
RANGE (IN.)	А	В	С	D	
3 1/2" – 4 1/2"	7"	6 1/8"	4 1/4"	6"	
5 3/8" – 7"	12"	10 7/8"	6 1/2"	8"	
7" – 8 1/2"	12"	10 7/8"	8"	8"	
8 3/16" – 9 3/4"	12"	10 7/8"	9 1/4"	8"	
9 1/4" - 11"	16"	14 7/8"	10 1/2"	8"	
10 1/4" - 11"	16"	14 7/8"	12"	8"	
12" – 13 3/4"	16"	14 7/8"	13 1/4"	8"	
14 1/2" – 16 1/4"	20"	18 7/8"	15 3/4"	8"	
15 3/4" – 17 1/2"	20"	18 7/8"	17"	8"	
19 1/2" – 21 1/4"	24"	22 7/8"	20 3/4"	8"	

CONCENTRIC MANHOLE DETAIL

NOT TO SCALE

LOCATION MAP:
R 3 W
T WINTH ST PROJECT LOCATION N SW 134TH ST
NOT TO SCALE
PROJECT: HIGHLAND WEST JR. HIGH 901 N. SANTA FE MOORE, OK
PROJECT NUMBER: 23069 DRAWING DATE: 11.02.23 ISSUE DATE: 11.02.23
SEAL:
SUBMITTAL: PERMIT SET
REVISIONS:

DRAWING TITLE:

SHEET:

⚠ 11.02.23 CB #1

THESE PLANS AND DRAWINGS ARE NOT TO BE REPRODUCED, CHANGED OR COPIED IN ANY FORM OR MANNER WHATSOEVER WITHOUT FIRST OBTAINING THE WRITTEN PERMISSION AND CONSENT

OF CEDAR CREEK CONSULTING INC. THIS SHEET IS NOT TO BE USED FOR CONSTRUCTION UNLESS THE ISSUE DATE IN THE TITLE BLOCK COINCIDES WITH OR

POST DATES THE DRAWING DATE. ANY CHANGES MADE FROM THESE PLANS WITHOUT CONSENT OF CEDAR CREEK CONSULTING INC. ARE

UNAUTHORIZED, AND SHALL RELIEVE CEDAR CREEK CONSULTING OF RESPONSIBILITY FOR ALL CONSEQUENCES ARRIVING OUT OF SUCH CHANGES.

STANDARD

DETAILS

C6.01

DETENTION POND PLANS TO SERVE HIGHLAND WEST JR HIGH CLASSROOM ADDITION

SHEET NUMBER	SHEET TITLE	DATE	REV/BID/CO/ADI
C0.00	DETENTION COVER SHEET	11.02.23	BID/CB #1
C3.00	GRADING PLAN	11.02.23	BID/CB #1
C3.01	DRAINAGE - HISTORIC	11.02.23	BID/CB #1
C 3.02	DRAINAGE - DEVELOPED	11.02.23	BID/CB #1
C 3.03	DETENTION POND PLAN	11.02.23	BID/CB #1
C 5.00	EROSION CONTROL PLAN	11.02.23	BID/CB #1
C 5.01	EROSION CONTROL NOTES	09.25.23	

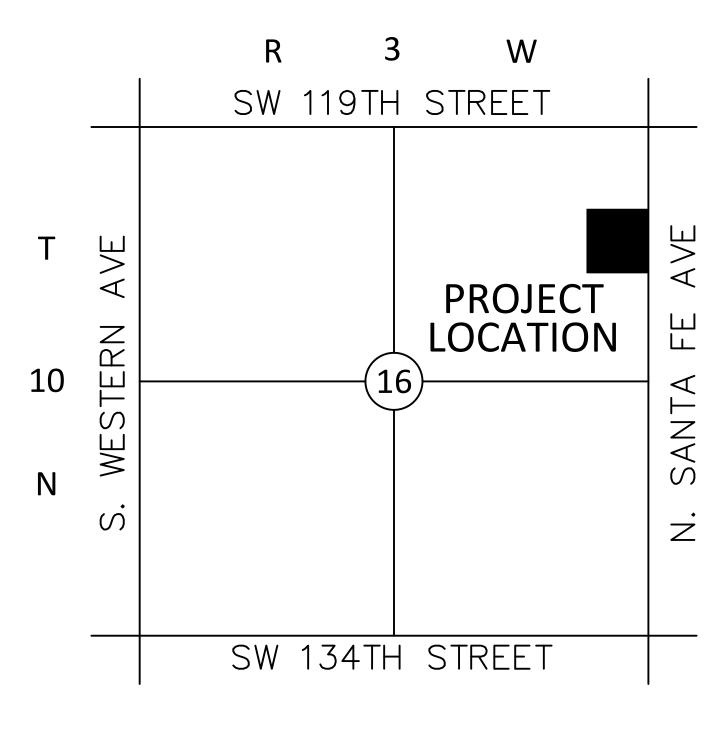
BENCHMARK DATA

BENCHMARK #1 DESC: CONC. SIDEWALK NORTHING: 732420.67 EASTING: 2113951.32 ELEVATION: 1246.12

BENCHMARK #2 DESC: CUT X NORTHING: 732831.70 EASTING: 2113951.35 ELEVATION: 1248.00

VERTICAL DATUM: NAVD 88 OKC GPS MONUMENT

A PART OF THE NE/4 OF SEC. 16, T-10-N, R-3-W, I.M. MOORE, CLEVELAND COUNTY, OKLAHOMA







GENERAL NOTES:

- CONTEMPORARY BASIS AT THE SITE.
- CONSTRUCTION SITE.
- ENGINEER.

A. CONTRACTOR SHALL BE RESPONSIBLE FOR RAZING AND REMOVAL OF THE EXISTING STRUCTURES, RELATED UTILITIES, PAVING, UNDERGROUND STORAGE TANKS AND ANY OTHER EXISTING IMPROVEMENTS AS NOTED.

B. CONTRACTOR IS TO REMOVE AND DISPOSE OF ALL DEBRIS, RUBBISH AND OTHER MATERIALS RESULTING FROM PREVIOUS AND CURRENT DEMOLITION OPERATIONS. DISPOSAL WILL BE IN ACCORDANCE WITH ALL LOCAL, STATE AND/OR FEDERAL REGULATIONS GOVERNING SUCH OPERATIONS.

C. THE GENERAL CONTRACTOR WILL BE HELD SOLELY RESPONSIBLE FOR AND SHALL TAKE ALL PRECAUTIONS NECESSARY TO AVOID PROPERTY DAMAGE TO ADJACENT PROPERTIES DURING THE CONSTRUCTION PHASES OF THIS PROJECT.

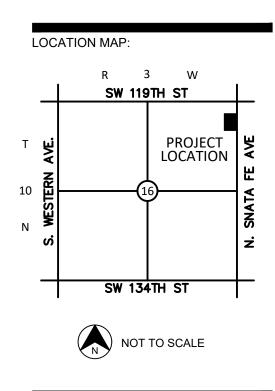
D. WARRANTY/DISCLAIMER: THE DESIGNS REPRESENTED IN THESE PLANS ARE IN ACCORDANCE WITH ESTABLISHED PRACTICES OF CIVIL ENGINEERING FOR THE DESIGN FUNCTIONS AND USES INTENDED BY THE OWNER AT THIS TIME. HOWEVER, NEITHER THE ENGINEER NOR ITS PERSONNEL CAN OR DO WARRANT THESE DESIGNS OR PLANS AS CONSTRUCTED EXCEPT IN THE SPECIFIC CASES WHERE THE ENGINEER INSPECTS AND CONTROLS THE PHYSICAL CONSTRUCTION ON A

E. SAFETY NOTICE TO CONTRACTOR: IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY DURING PERFORMANCE OF THE WORK. THIS REQUIREMENT WILL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS. ANY CONSTRUCTION OBSERVATION BY THE ENGINEER OF THE CONTRACTOR'S PERFORMANCE IS NOT INTENDED TO INCLUDE REVIEW OF THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES, IN, ON OR NEAR THE

F. ALL CONSTRUCTION WITHIN STATE HIGHWAY DEPARTMENT RIGHT-OF-WAY SHALL BE COORDINATED WITH THE HIGHWAY DEPARTMENT RESIDENT MAINTENANCE

G. ALL CONSTRUCTION TO BE IN STRICT ACCORDANCE WITH CURRENT CITY OF MOORE STANDARDS AND SPECIFICATIONS





PROJECT:

HIGHLAND WEST JR. HIGH

901 N. SANTA FE MOORE, OK

PROJECT NUMBER DRAWING DATE: **ISSUE DATE:**

11.02.23 11.02.23



SUBMITTAL:

PERMIT SET

REVISIONS: 11.02.23

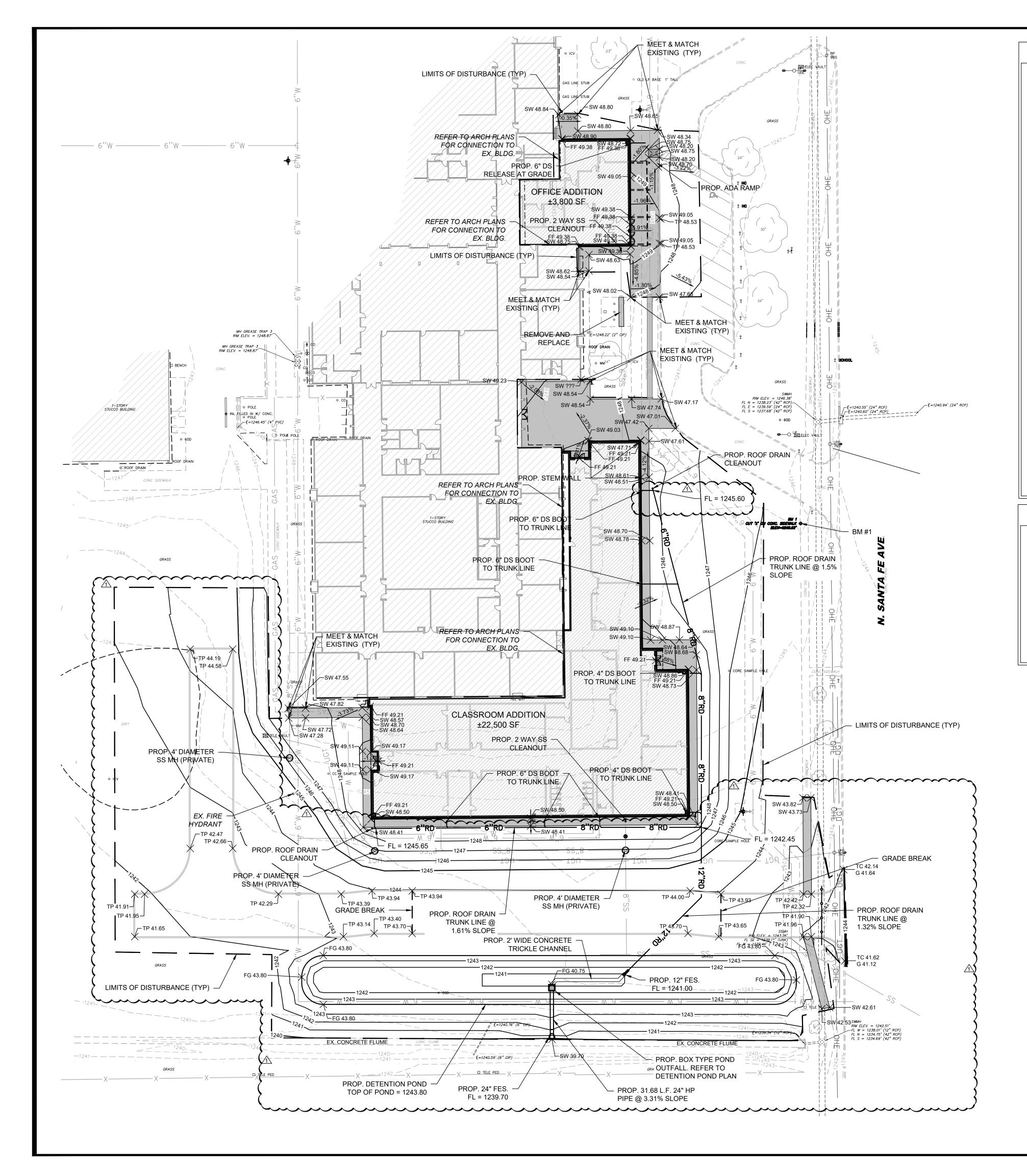
CB #1

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DRAWING TITLE:







LEGE	INE)
BOUNDARY RIGHT OF W EASEMENT EASEMENT EASEMENT EXISTING CO PROPOSED PROPOSED OHE OVERHEAD UGE UNDERGRO GAS GAS LINE UNDERGRO SS SANITARY S 8"W WATERLINE BENCHMARI	AY L LINE DNC CON FIRE ELE UND UND EWE	INE RETE CUF ICRETE C ELANE ST CTRIC ELECTRIC TELEPHC FIBER OF
	Ø	EX. POW
WATER VALVE	ø	PROP. P
EX. WATER METER PIT	TPL	EX. TELE
⊘ EX. WATER METER	(T)	EX. TELE
PROP. WATER METER	TSL 口	EX. TRAF
⊚ ^{SCV} EX. SPRINKLER VALVE	TCE	, EX. TRAF
© EX. AUTO SPRINKLER	0 FP	EX. FLAG
□ EX. ELECT. PEDESTAL	OYL	EX. YARI
\square^{ET} EX. ELECT. TRANSFORMER	6	EX. GRE
□ EX. ELECT. METER	S	EX. SS M
T PROP. ELECT. METER	S	PROP. S
\square^{AC} EX. AIR CONDITIONER	Ø	EX. GAS
EX. SIGNAGE		PROP. G
₩ EX. LIGHT POLE	P	EX. ELEC
GE PROP. LIGHT POLE	S	EX. STO
e ^{GP} EX. BOLLARD		
	-	

BENCHMARK DAT

BENCHMARK #1 DESC: CONC. SIDEWALK NORTHING: 732420.67 EASTING: 2113951.32 ELEVATION: 1246.12

BENCHMARK #2 DESC: CUT X NORTHING: 732831.70 EASTING: 2113951.35 ELEVATION: 1248.00

VERTICAL DATUM: NAVD 88 OKC GPS MONUMENT

	GRADING NOTES
	A. CONTRACTOR SHALL REFER TO THE SITE SPECIFIC GEOTECHNICAL REPORT FOR EXISTING SOIL CONDITIONS, CONSIDERATIONS, AND RECOMMENDATIONS.
E CURB AND GUTTER TE CURB AND GUTTER IE STRIPING C CTRIC	B. CONTRACTOR SHALL REFER TO THE CONSTRUCTION DOCUMENTS INCLUDING BUT NOT LIMITED TO THE WRITTEN SPECIFICATIONS, CONSTRUCTION DRAWINGS, STORM WATER POLLUTION PLAN, AND GEOTECHNICAL REPORT.
EPHONE ER OPTIC	C. CONTRACTOR IS RESPONSIBLE FOR THEIR OWN HORIZONTAL AND VERTICAL CONTROL, REFERENCE POINTS AND CONSTRUCTION STAKING AS INCIDENTAL TO THE PROJECT.
	D. THE CONTRACTOR SHALL FIELD VERIFY EXISTING ELEVATIONS/PROPERTY LINES/UTILITIES/DRAINAGE PRIOR TO CONSTRUCTION START.
POWER POLE P. POWER POLE TELEPHONE PED.	E. ALL SITE EXCAVATION SHALL BE CONSIDERED UNCLASSIFIED EXCAVATION.
TELEPHONE MANHOLE	F. GENERAL CONTRACTOR TO PROVIDE A UNIT PRICE FOR REMOVAL AND REPLACEMENT OF SOILS ON THIS SITE SHOULD REMOVAL BE REQUIRED.
TRAFFIC CONTROL BOX FLAG POLE YARD LIGHT GREASE TRAP	G. ALL WORK NOT CLASSIFIED AS A CONTRACT PAY ITEM SHALL BE CONSIDERED AS INCIDENTAL AND THE COST THEREOF SHALL BE INCLUDED IN THE UNIT PRICE BID FOR ITEMS WHICH ARE CLASSIFIED FOR PAYMENT.
SS MANHOLE DP. SS MANHOLE GAS METER	H. CONTRACTOR SHALL PROVIDE FINAL GRADES THAT DO NOT OBSTRUCT ANY UTILITY ACCESS AND PROVIDE A SMOOTH TRANSITION TO MEET AND MATCH EXISTING GRADES ON ALL SIDES.
DP. GAS METER ELECT. MANHOLE STORM MANHOLE	I. ADA ROUTES ARE NOT TO EXCEED 1:20 RUNNING SLOPE AND 2% CROSS SLOPE. HANDICAP PARKING AND ACCESS AISLES SHALL NOT EXCEED 2% IN ANY DIRECTION.
RADING PLAN FOR TYPE)	J. ALL NATURAL GROUND SLOPES SHALL NOT EXCEED 3:1. PAVING SLOPES SHALL NOT EXCEED 8%.
1ENT	K. CONTRACTOR SHALL ENSURE THAT ALL NECESSARY EARTH DISTURBING PERMITS HAVE BEEN ACQUIRED AND MEET THE CONDITIONS/REQUIREMENTS SET FORTH IN THE PERMITS PRIOR TO CONSTRUCTION.
ATA	L. CONTRACTOR IS REQUIRED TO CALL ONE CALL AS WELL AS THE APPROPRIATE UTILITY COMPANY AT LEAST 48 HOURS BEFORE ANY EXCAVATION/CONSTRUCTION ACTIVITIES TAKE PLACE. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH ARE IN CONFLICT WITH PROPOSED IMPROVEMENTS.
	M. THE CONTRACTOR SHALL GRADE SITE TO ENSURE ALL SURFACE WATER DRAINAGE IS AWAY FROM THE BUILDING AN PROVIDES POSITIVE DRAINAGE SO THAT NO STANDING/PONDING WATER TAKES PLACE ON SITE OR ON ADJACENT PROPERTIES.
	N. ALL CONSTRUCTION SHALL BE IN STRICT ACCORDANCE WITH THE OWNERS DESIGN GUIDELINES AND SPECIFICATIONS, AND WHERE APPLICABLE SHALL MEET THE REQUIREMENTS OF THE GOVERNING/PERMITTING AUTHORITY HAVING JURISDICTION.
	O. THE BUILDING SUBGRADE SHALL BE CONSTRUCTED TO INCLUDE A MINIMUM OF 10 FEET BEYOND THE BUILDING LIMITS AS SHOWN ON THE PLANS, OR AS DIRECTED BY THE OWNER.
	P. REFERENCE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR REQUIRED FLOOR SLAB THICKNESS.
	Q. THE BUILDING PAD SUBGRADE SHALL BE PREPARED IN STRICT ACCORDANCE WITH THE GEOTECHNICAL ENGINEERING STUDT AND THE CIVIL SPECIFICATIONS.
	R. ESTABLISH FINAL SUBGRADE ELEVATIONS TO ALLOW FOR PAVEMENT/SLAB SECTIONS AS INDICATED ON THE PLANS.
	S. IF CONFLICTS EXIST BETWEEN THE GEOTECHNICAL REPORT AND THE CONSTRUCTION DRAWINGS AND SPECIFICATIONS, THE MORE STRINGENT REQUIREMENTS SHALL APPLY.
	SPOT ELEVATION LEGEND
	TC - TOP OF CURB FF - FINISH FLOOR

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ENGINEERING • PLANNING • CONSULTING P.O. Box 14534 Oklahoma City, OK 73113

405-778-3385

www.cedarcreekinc.com

R 3 W SW 119TH ST

SW 134TH ST

NOT TO SCALE

HIGHLAND WEST

JR. HIGH

901 N. SANTA FE

MOORE, OK

23069

11.02.23

11.02.23

PROJECT NUMBER:

andren

ANDREW

WILSON

Wilson 24273

PERMIT SET

CB #1

DRAWING DATE:

ISSUE DATE:

SFAL

SUBMITTAL:

REVISIONS:

11.02.23

PROJECT:

PROJECT

LOCATION MAP:

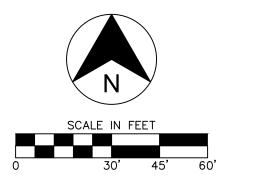
OK CA 5864 EXP. 06/30/24

DRAWING TITLE:

SHEET:

GRADING PLAN

C3.00



VS VERTICAL SEPARATION REQUIREMENT

TC - TOP OF CURB

TP - TOP OF PAVEMENT

G - GUTTER

HP - HIGH POINT

LP - LOW POINT

SW - SIDEWALK



FF - FINISH FLOOR

FG - FINAL GRADE

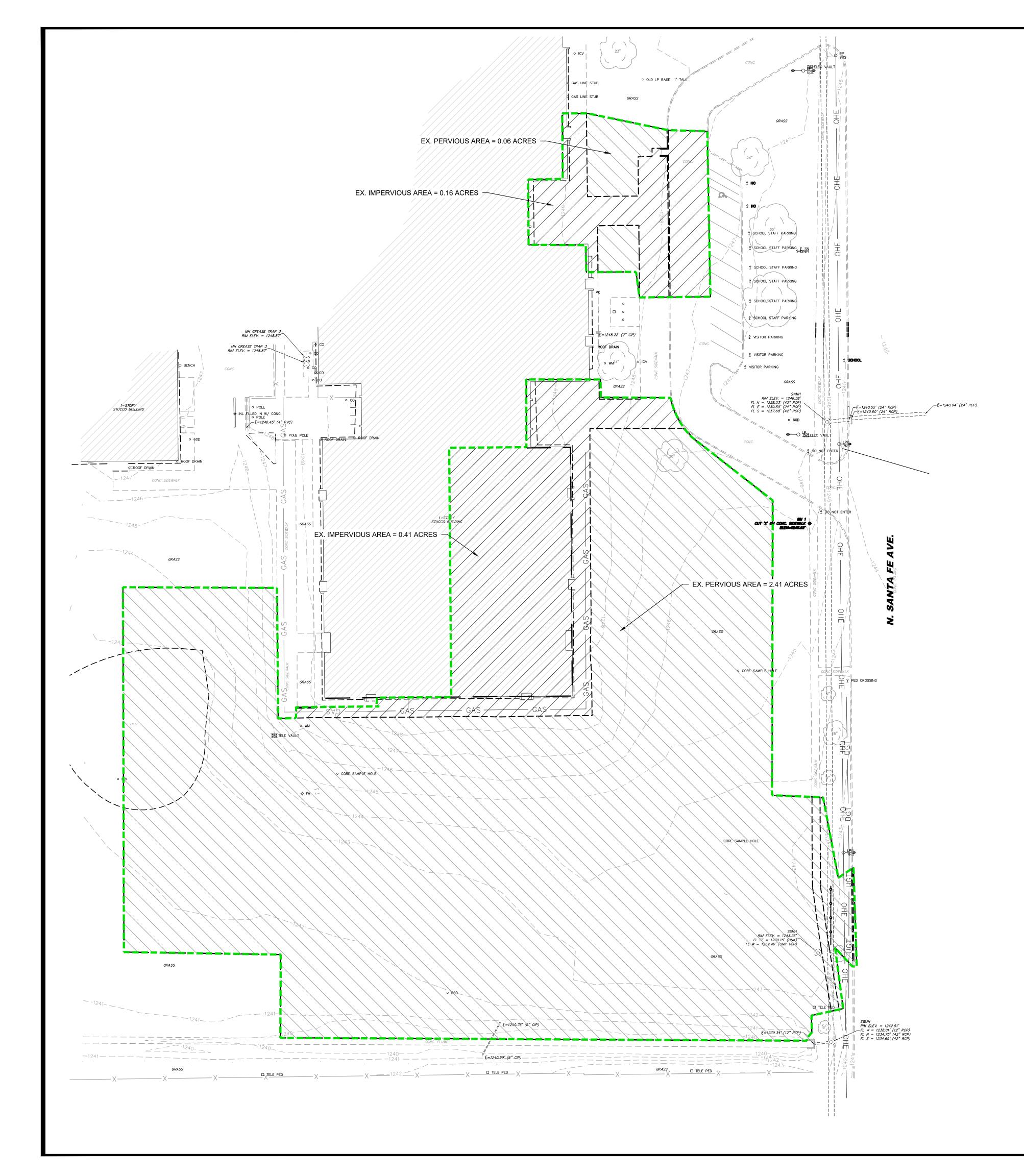
TW - TOP OF WALL

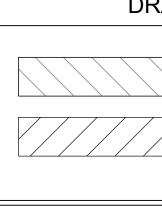
FOOTING

BW - BOTTOM OF WALL

NOTE: BW IS BOTTOM OF

WALL AT GRADE, NOT



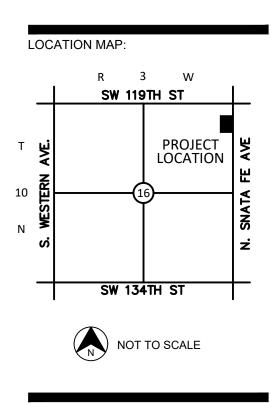


DRAINAGE LEGEND

EXISTING PERVIOUS CN = 80

EXISTING IMPERVIOUS CN = 98





PROJECT:

HIGHLAND WEST JR. HIGH

901 N. SANTA FE MOORE, OK	
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PROJECT NUMBER:	23069
DRAWING DATE:	11.02.23
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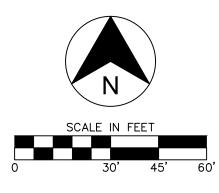
REVISIONS: <u> 11.02.23</u>

CB #1

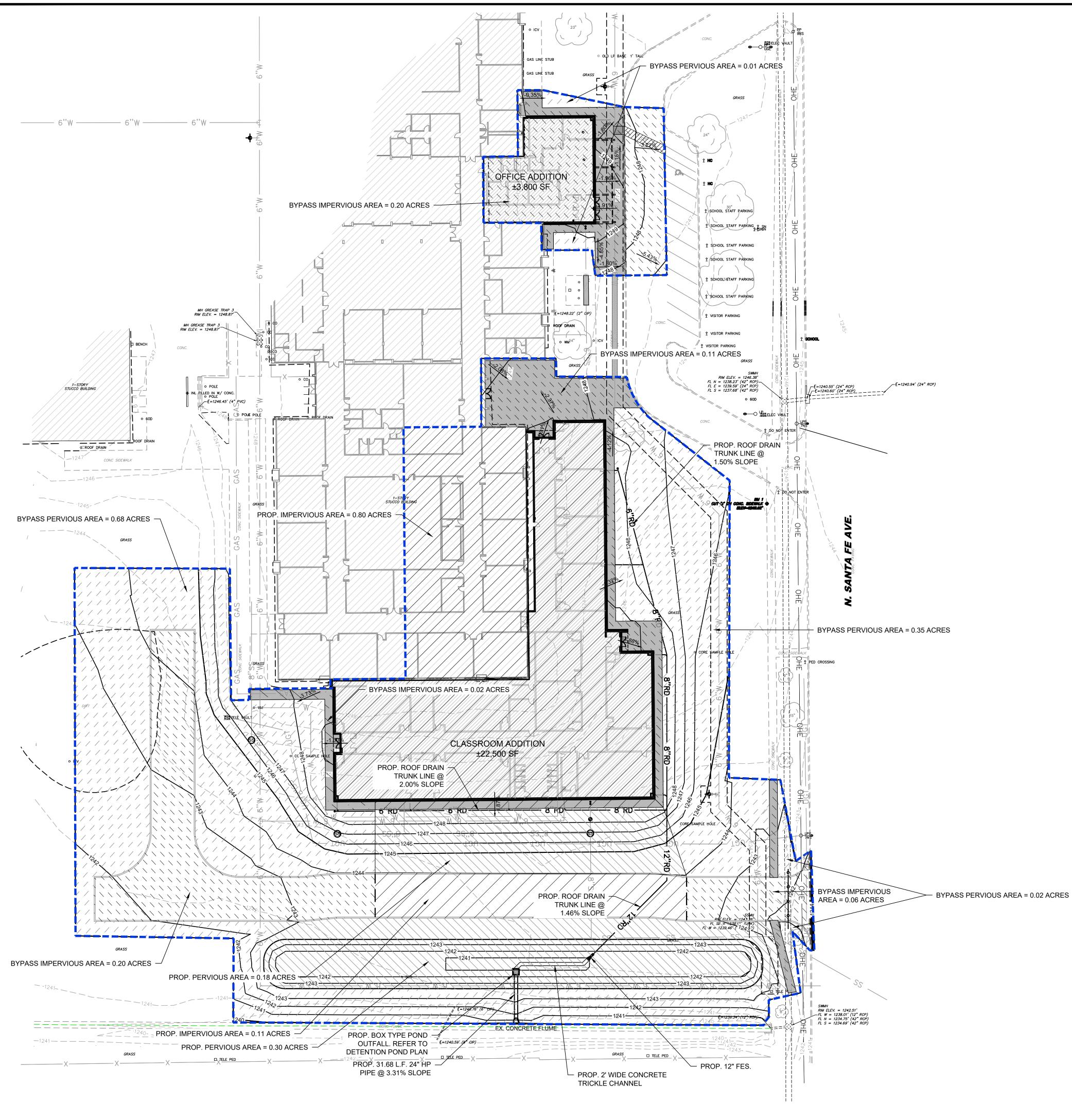


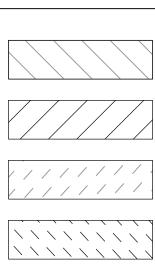


mmmm









DRAINAGE LEGEND

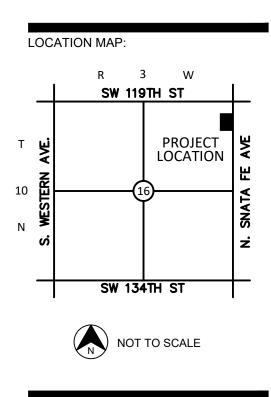
PROPOSED PERVIOUS CN = 80

PROPOSED IMPERVIOUS CN = 98

BYPASS IMPERVIOUS CN = 80

BYPASS IMPERVIOUS CN = 98





PROJECT:

HIGHLAND WEST JR. HIGH

901 N. SANTA FE MOORE, OK

PROJECT NUMBER:	
DRAWING DATE:	1
ISSUE DATE:	1

23069 11.02.23 11.02.23

SEAL andren ANDREW WILSON Wilson 24273

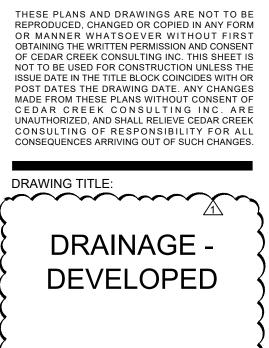
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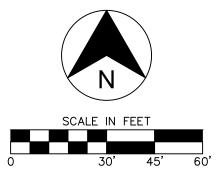
CB #1

SHEET:

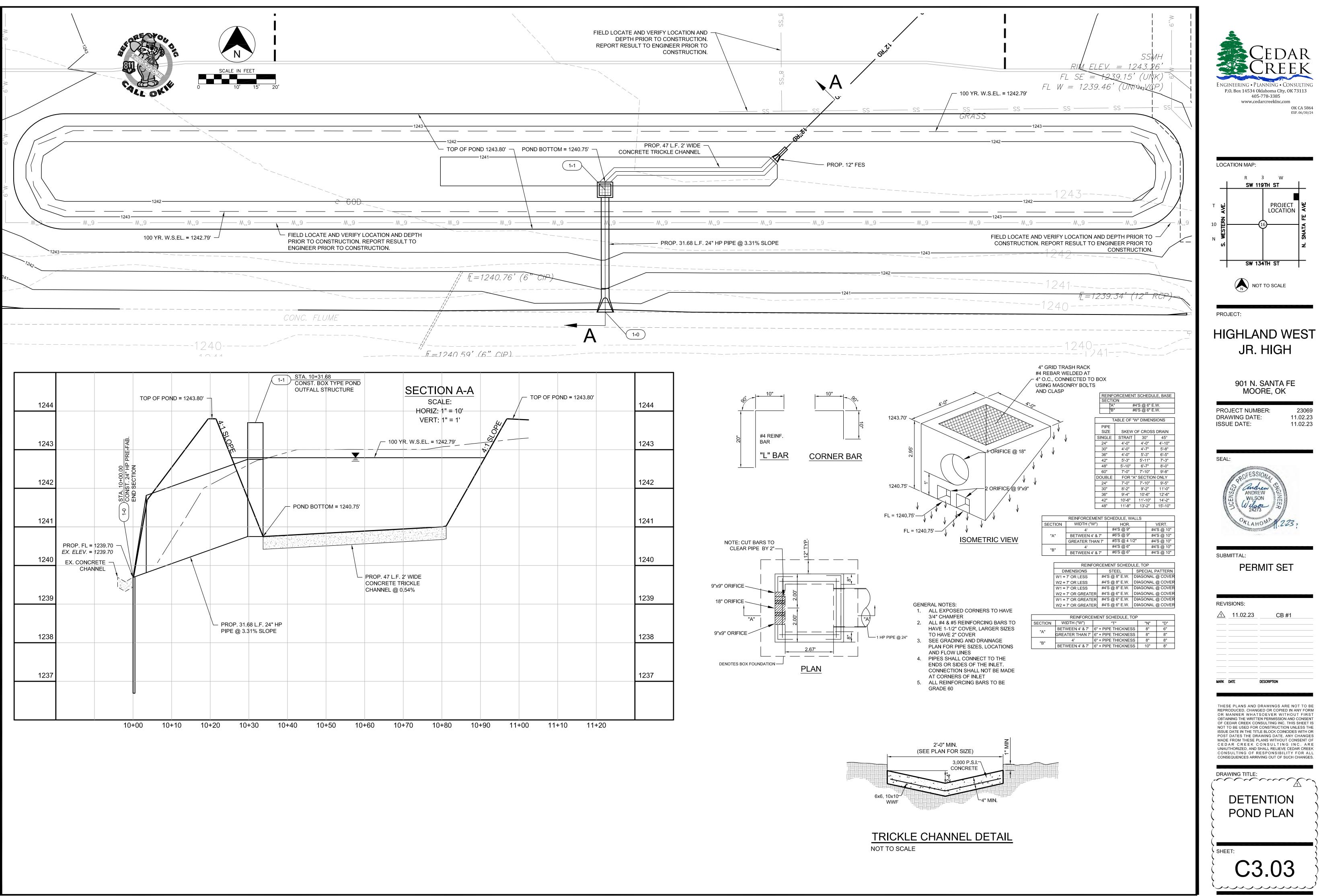


C3.02

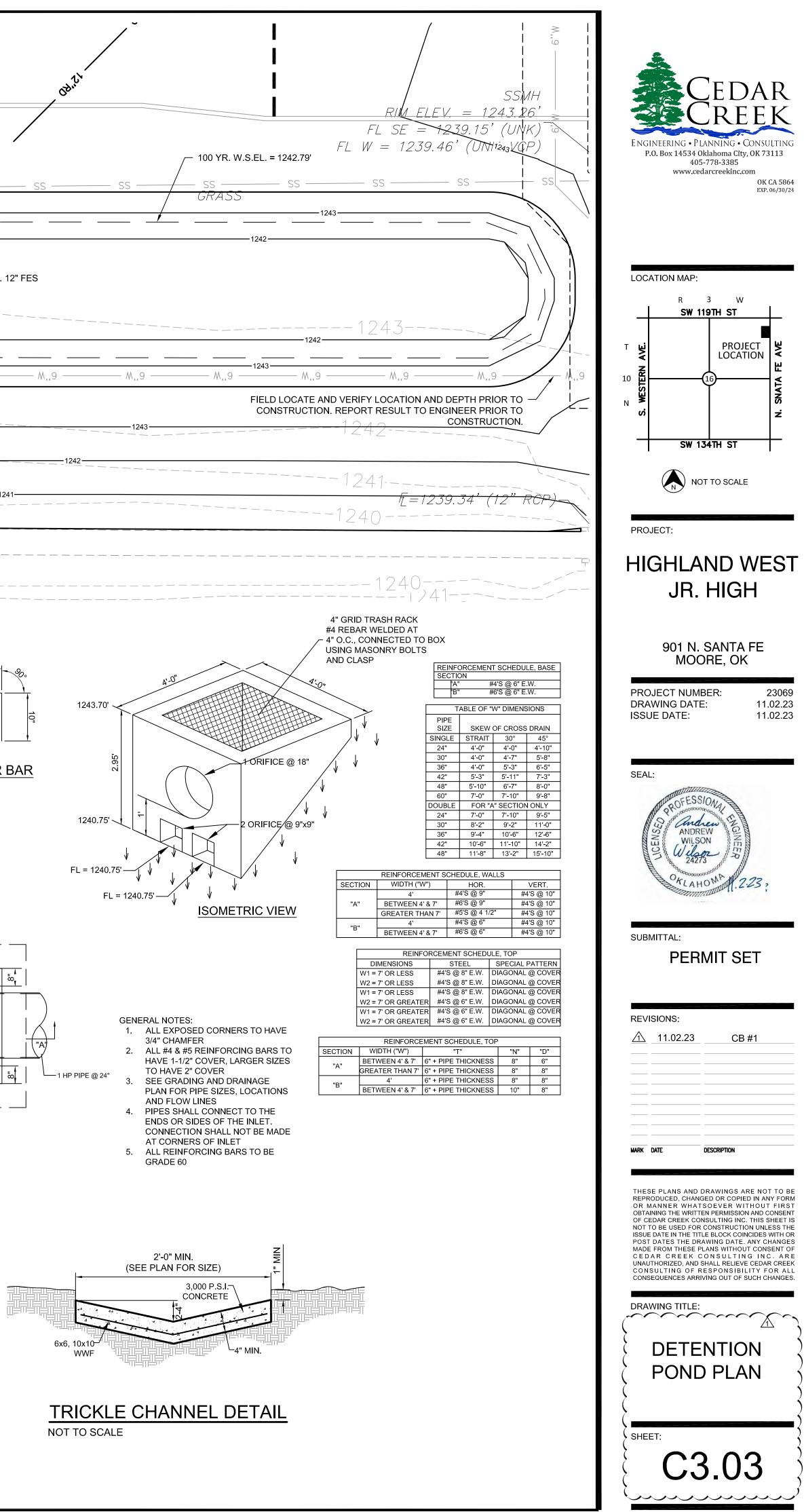
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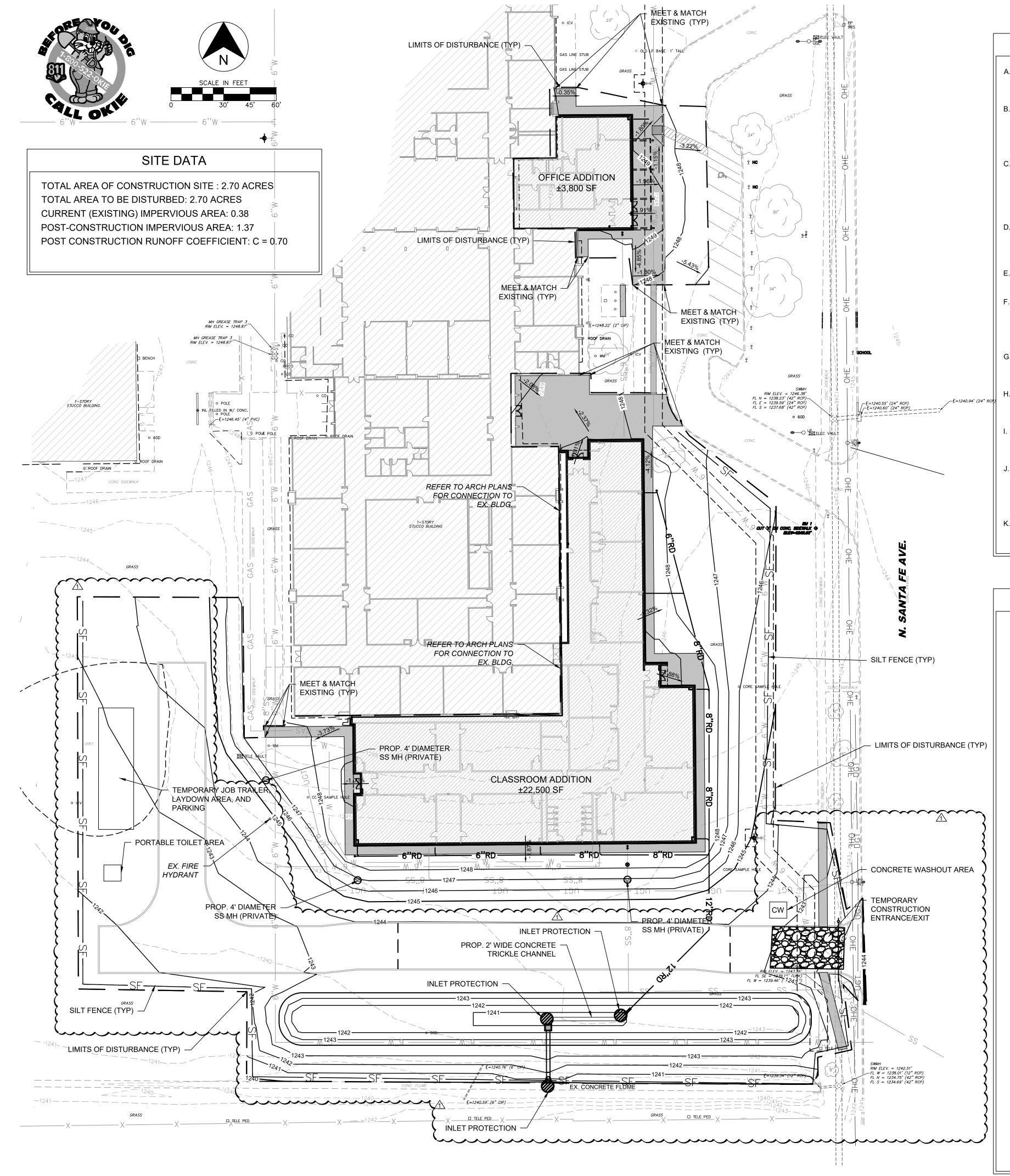






TION A-A SCALE: RIZ: 1" = 10'	/		DND = 1243.80'		1244
ERT: 1" = 1'					
= 1242.79'	5				1243
	7				
					1242
					1241
					1240
					1239
					1238
					1237
10+80 10-	+90 11+	-00 11-	-10 11-	+20	





A. SEDIMENT BASINS ARE ATTRACTIVE TO CHILDREN AND CAN BE VERY DANGEROUS. IN ALL CASES, LOCAL ORDINANCES AND REGULATIONS

REGARDING HEALTH AND SAFETY MUST BE ADHERED TO.

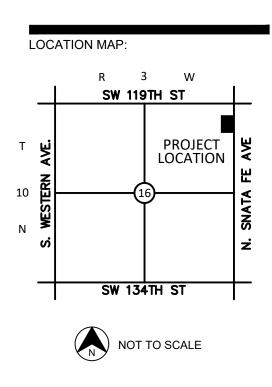
- B. ALL CONTRACTORS AND SUBCONTRACTORS INVOLVED WITH STORM WATER POLLUTION PREVENTION SHALL OBTAIN A COPY OF THE STORM WATER POLLUTION PREVENTION PLAN AND THE STATE OF OKLAHOMA NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM GENERAL PERMIT (NPDES PERMIT) AND BECOME FAMILIAR WITH THEIR CONTENTS.
- C. ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHALL BE DISPOSED OF WITHIN 30 DAYS AFTER FINAL STABILIZATION. FINAL STABILIZATION HAS OCCURRED WHEN ALL SOIL DISTURBING ACTIVITIES ARE COMPLETED AND A UNIFORM PERENNIAL VEGETATIVE COVER WITH A DENSITY OF 70% OF THE COVER FOR UNPAVED AREAS AND AREAS NOT COVERED BY PERMANENT STRUCTURES HAS BEEN EMPLOYED.
- D. BEST MANAGEMENT PRACTICES (BMP'S) AND CONTROLS SHALL CONFORM TO FEDERAL, STATE, OR LOCAL REQUIREMENTS OR MANUAL OF PRACTICE, AS APPLICABLE CONTRACTOR SHALL IMPLEMENT ADDITIONAL CONTROLS AS DIRECTED BY PERMITTING AGENCY OR OWNER.
- . CONTRACTOR SHALL MINIMIZE CLEARING TO THE MAXIMUM EXTENT PRACTICAL OR AS REQUIRED BY THE GENERAL PERMIT.
- GENERAL CONTRACTOR SHALL DENOTE ON PLAN THE TEMPORARY PARKING AND STORAGE AREA WHICH SHALL ALSO BE USED AS THE EQUIPMENT MAINTENANCE AND CLEANING AREA, EMPLOYEE PARKING AREA, AND AREA FOR LOCATING PORTABLE FACILITIES, OFFICE TRAILERS, AND TOILET FACILITIES.
- G. ALL WASH WATER (CONCRETE TRUCKS, VEHICLE CLEANING, EQUIPMENT CLEANING, ETC.) SHALL BE DETAINED AND PROPERLY TREATED OR DISPOSED.
- H. SUFFICIENT OIL AND GREASE ABSORBING MATERIALS AND FLOTATION BOOMS SHALL BE MAINTAINED ON SITE OR READILY AVAILABLE TO CONTAIN AND CLEAN-UP FUEL OR CHEMICAL SPILLS AND LEAKS.
- DUST ON THE SITE SHALL BE CONTROLLED. THE USE OF MOTOR OILS AND OTHER PETROLEUM BASED OR TOXIC LIQUIDS FOR DUST SUPPRESSION OPERATIONS IS PROHIBITED.
- RUBBISH, TRASH, GARBAGE, LITTER, OR OTHER SUCH MATERIALS SHALL BE DEPOSITED INTO SEALED CONTAINERS. MATERIALS SHALL BE PREVENTED FROM LEAVING THE PREMISES THROUGH THE ACTION OF WIND OR STORMWATER DISCHARGE INTO DRAINAGE DITCHES OR WATERS OF THE STATE.
- K. ALL STORM WATER POLLUTION PREVENTION MEASURES PRESENTED ON THIS PLAN, AND IN THE STORM WATER POLLUTION PREVENTION PLAN, SHALL BE INITIATED AS SOON AS PRACTICABLE.

EROSION CONTROL NOTES

L. DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS STOPPED FOR AT LEAST 14 DAYS, SHALL BE TEMPORARILY SEEDED. THESE AREAS SHALL BE SEEDED NO LATER THAN 14 DAYS FROM THE LAST CONSTRUCTION ACTIVITY OCCURRING IN THESE AREAS.

- M. DISTURBED PORTIONS OF THE SITE WHERE CONSTRUCTION ACTIVITY HAS PERMANENTLY STOPPED SHALL BE PERMANENTLY SEEDED. THESE AREAS SHALL BE SEEDED NO LATER THAN 14 DAYS AFTER THE LAST CONSTRUCTION ACTIVITY OCCURRING IN THESE AREAS. REFER TO THE GRADING PLAN AND/OR LANDSCAPE PLAN.
- N. IF THE ACTION OF VEHICLES TRAVELING OVER THE GRAVEL CONSTRUCTION ENTRANCES IS NOT SUFFICIENT TO REMOVE THE MAJORITY OF DIRT OR MUD, THEN THE TIRES MUST BE WASHED BEFORE THE VEHICLES ENTER A PUBLIC ROAD. IF WASHING IS USED, PROVISIONS MUST BE MADE TO INTERCEPT THE WASH WATER AND TRAP THE SEDIMENT BEFORE IT IS CARRIED OFF THE SITE.
- O. ALL MATERIALS SPILLED, DROPPED, WASHED, OR TRACKED FROM VEHICLES ONTO ROADWAYS OR INTO STORM DRAINS MUST BE REMOVED IMMEDIATELY.
- P. CONTRACTORS OR SUBCONTRACTORS WILL BE RESPONSIBLE FOR REMOVING SEDIMENT IN THE DETENTION POND AND ANY SEDIMENT THAT MAY HAVE COLLECTED IN THE STORM SEWER DRAINAGE SYSTEMS IN CONJUNCTION WITH THE STABILIZATION OF THE SITE.
- Q. ON-SITE & OFFSITE SOIL STOCKPILE AND BORROW AREAS SHALL BE PROTECTED FROM EROSION AND SEDIMENTATION THROUGH IMPLEMENTATION OF BEST MANAGEMENT PRACTICES. STOCKPILE AND BORROW AREA LOCATIONS SHALL BE NOTED ON THE SITE PLAN AND PERMITTED IN ACCORDANCE WITH GENERAL PERMIT REQUIREMENTS.
- R. SLOPES SHALL BE LEFT IN A ROUGHENED CONDITION DURING THE GRADING PHASE TO REDUCE RUNOFF VELOCITIES AND EROSION.
- S. DUE TO THE GRADE CHANGES DURING THE DEVELOPMENT OF THE PROJECT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADJUSTING THE EROSION CONTROL MEASURES (SILT FENCES, STRAW BALES, ETC.) TO PREVENT EROSION.
- T. ALL CONSTRUCTION SHALL BE STABILIZED AT THE END OF EACH WORKING DAY, THIS INCLUDES BACKFILLING OF TRENCHES FOR UTILITY CONSTRUCTION AND PLACEMENT OF GRAVEL OR BITUMINOUS PAVING FOR ROAD CONSTRUCTION.
- U. A 3' STRIP OF SOD SHALL BE PLACED ALONG THE EDGE OF ALL PAVING TO ACT AS A SEDIMENT BUFFER AND AID IN THE ESTABLISHMENT OF VEGETATION.





PROJECT:

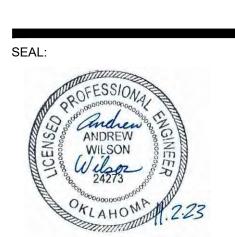
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ISSUE DATE:

HIGHLAND WEST JR. HIGH

901 N. SANTA MOORE, OI	
OJECT NUMBER:	23069
AWING DATE:	11.02.23

11.02.23



SUBMITTAL:

PERMIT SET

REVISIONS: 11.02.23

CB #1

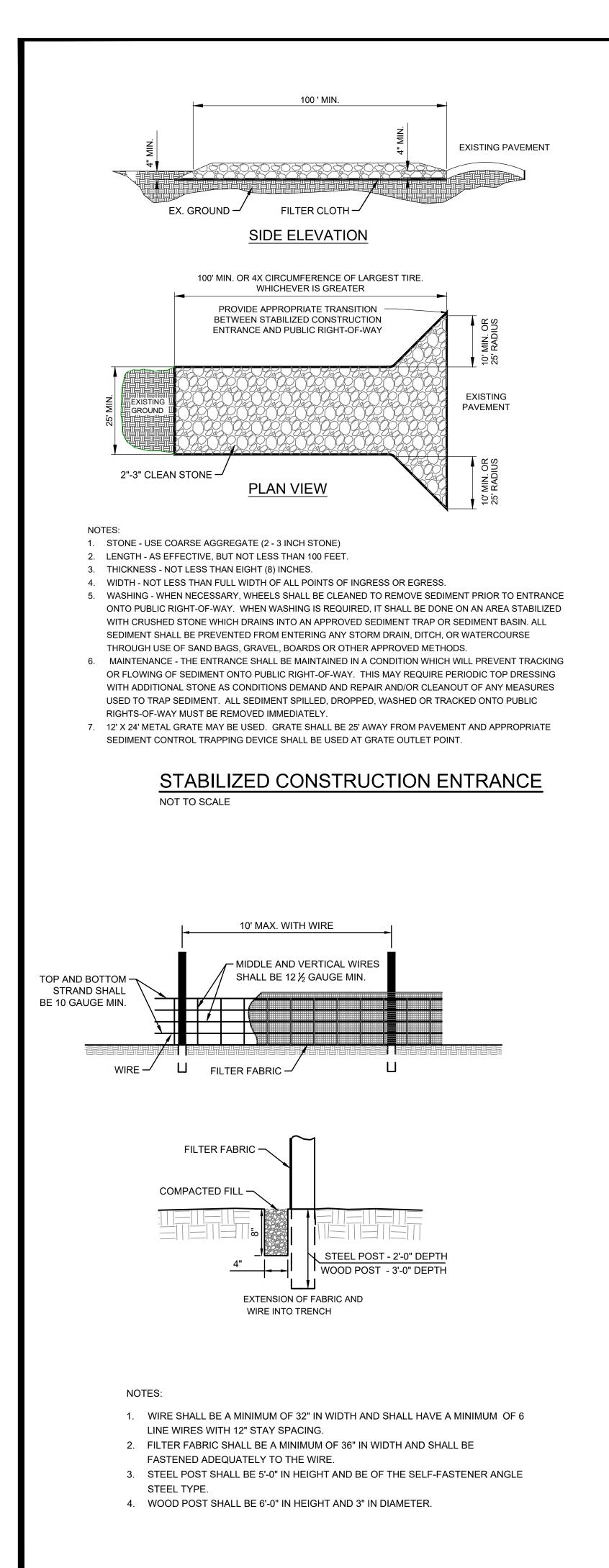
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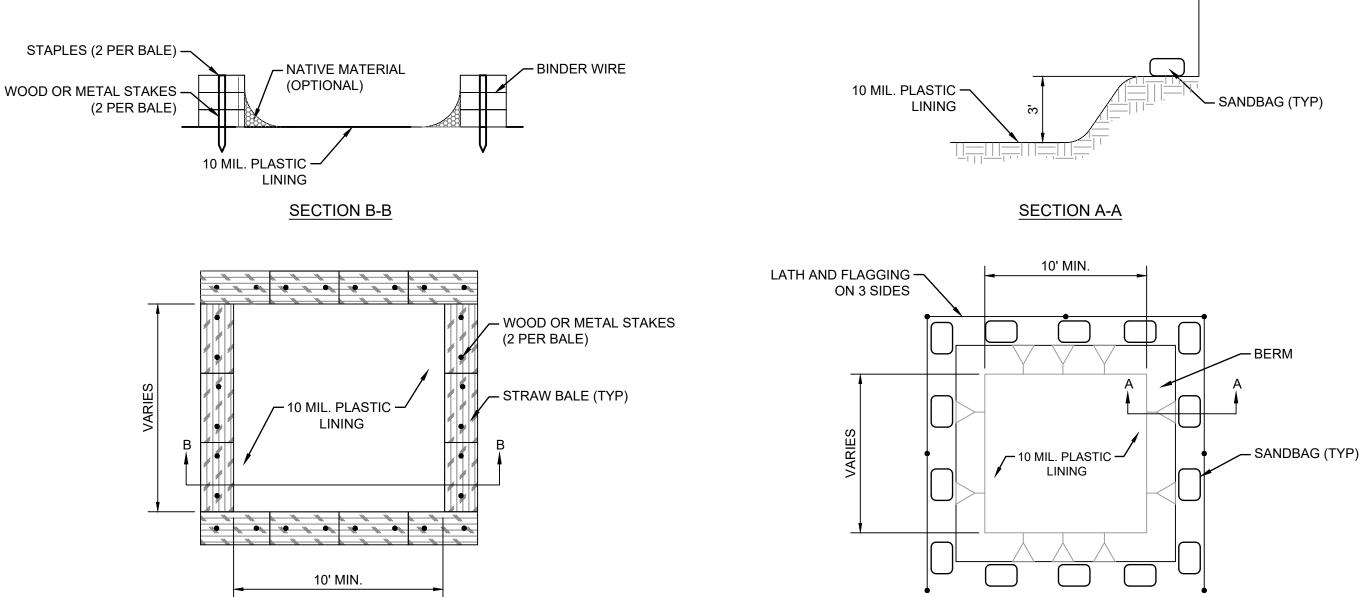
> EROSION CONTROL PLAN

C5.00

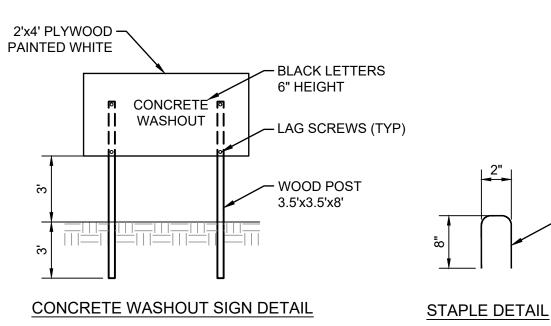
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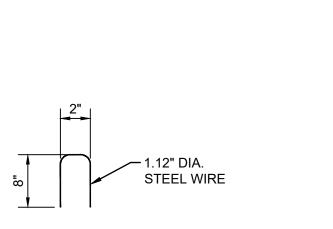


SILT FENCE DETAIL NOT TO SCALE



TYPE 'ABOVE GRADE' WITH STRAW BALES





NOTES:

- 1. ACTUAL LAYOUT TO BE DETERMINED IN THE FIELD.
- 2. A CONCRETE WASHOUT SIGN SHALL BE INSTALLED WITHIN 30' OF THE TEMPORARY CONCRETE WASHOUT FACILITY
- 3. MATERIALS USED TO CONSTRUCT TEMPORARY CONCRETE WASHOUT FACILITIES SHALL BE REMOVED FROM THE SITE OF THE WORK AND DISPOSED OF OR RECYCLED.
- 4. HOLES, DEPRESSIONS OR OTHER GROUND DISTURBANCE CAUSE BY THE REMOVAL OF THE TEMPORARY CONCRETE WASHOUT FACILITIES SHALL BE BACKFILLED, REPAIRED AND STABILIZED TO PREVENT EROSION.

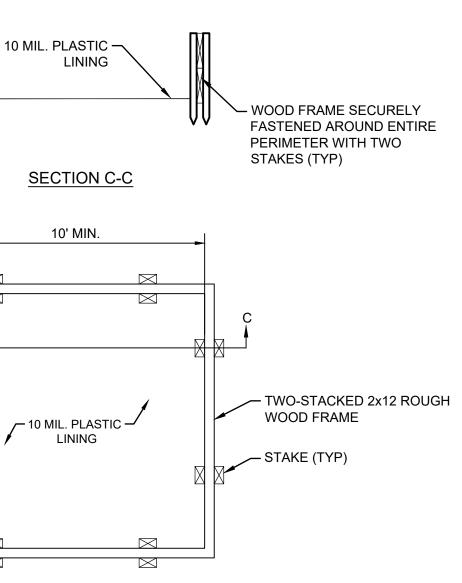
TYPE 'ABOVE GRADE' WITH WOOD PLANKS

 \bowtie

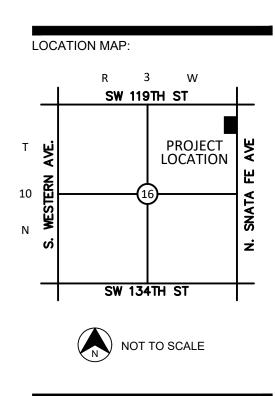
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CONCRETE WASHOUT DETAIL NOT TO SCALE

TYPE 'BELOW GRADE'







PROJECT:

HIGHLAND WEST JR. HIGH

901 N. SANT, MOORE, C	
ROJECT NUMBER:	23069
RAWING DATE:	11.02.23
SSUE DATE:	11.02.23

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SUBMITTAL:

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REVISIONS: 11.02.23

CB #1

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CONSEQUENCES ARRIVING OUT OF SUCH CHANGES.

DRAWING TITLE:



SHEET: C5.01

STORM WATER DRAINAGE COMPOSITION REPORT

For

Highland West Jr high Classroom Addition Moore, OK

Submitted July 26, 2023 Revised September 25, 2023 Revised November 2, 2023



Prepared by:



STORM WATER DRAINAGE COMPOSITION REPORT



July 26, 2023 Revised September 25, 2023 Revised November 2, 2023

Highland West Jr High Classroom addition Moore, OK Cedar Creek Project # 23069

PROJECT DESCRIPTION

The following is the Storm Water Drainage Composition Report for the construction of the proposed Highland West Jr high classroom addition in Moore, Oklahoma. The drainage area considered in this development is approximately 2.95 acres. This design and report is only accounting for the offset of the proposed additional impervious area.

This report will act as an accounting of the pond release in comparison to its current state, showing the total flow leaving the site will not increase as part of the development of the subject areas.

CURRENT SITE CONDITIONS

The existing subject area on site consists of 3.04 acres of currently developed school area. Of the 3.04 acres, approximately 0.57 acres is currently impervious. The remaining 2.47 acres is greater than 75% grass cover.

The runoff CN value for the site was taken as weighted developed area with Hydrologic Soil Group 'D'. With the Corresponding CN value of 80, the existing site releases stormwater according to the table below:

EXISTING FLOW TABLE

	CN	Area	Тс	Q100	Pond
		(ac)	(min)	(cfs)	Elevation
Existing Site	83	3.04	5	37.44	
*Q100 FROM HYDROCAD CALCULATIONS					
* TC used a min of 5 min.					

DEVELOPED CONDITIONS

The site will be developed to add additional classrooms and offices as well as an asphalt road. From the 3.04 acres, runoff from 1.39 acres will drain to the proposed detention pond, and runoff from 1.65 acres will be bypassing the detention pond. The detention pond will discharge to an existing trickle channel that runs west to east, at the southern portion of the site.

Per the new construction and the new detention pond, the flows are presented below:

Rainfall depth-duration estimates taken from City of Moore SMC table 3.

	Area	Тс	Peak Flow	Peak Pond	
	(acres)	(minutes)	(Q100)	Elevation	
Developed to Pond	1.39	5	18.49	1242.79'	
Release from Pond	1.39		11.56		
Bypass	1.65	5	20.96		
Total Release from Site	3.04		30.97		
(Pond Release + Bypass)					
Existing Site (from above)	3.04		37.44		
Change in Flow			-6.47 cfs		

COMPARISON OF PEAK FLOWS

The peak flow for the proposed development will be reduced to below the existing values via the proposed detention pond. The pond release will be controlled by a box type outfall with two 9"x9" orifices and an 18" diameter orifice. The pond release will then go through 24" HP Pipe to an existing trickle channel. Per the attached HydroCAD report, the impact on the onsite storage is summarized below (optimized for maximum pond volume).

			, ,	,		
STRM	Volume	Volume	% used	Q (cfs) (pond	Peak	Outfall Size and Type
	required	provided		out)	Elevation	
2	1,694	14,157	12	4.27	1241.76'	(2) 9"x9" orifices and
						(1) 18" diameter orifice
5	2,591	14,157	18	5.31	1241.99'	(2) 9"x9" orifices and
						(1) 18" diameter orifice
10	3,425	14,157	24	6.40	1242.17'	(2) 9"x9" orifices and
						(1) 18" diameter orifice
50	5,757	14,157	41	9.84	1242.60'	(2) 9"x9" orifices and
						(1) 18" diameter orifice
100	6,931	14,157	49	11.56	1242.79'	(2) 9"x9" orifices and
						(1) 18" diameter orifice
*FROM HYDROCAD CALCULATIONS						

DETENTION POND INFORMATION (100 year storm)



Conclusions

Based on the provided site data, the proposed adjustment to the drainage basins is not anticipated to have an adverse impact on capacity or competence of the downstream drainage facilities. Further, development of the site as proposed is in keeping with the intent of the approved design.



National Flood Hazard Layer FIRMette

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500

1,000

1,500

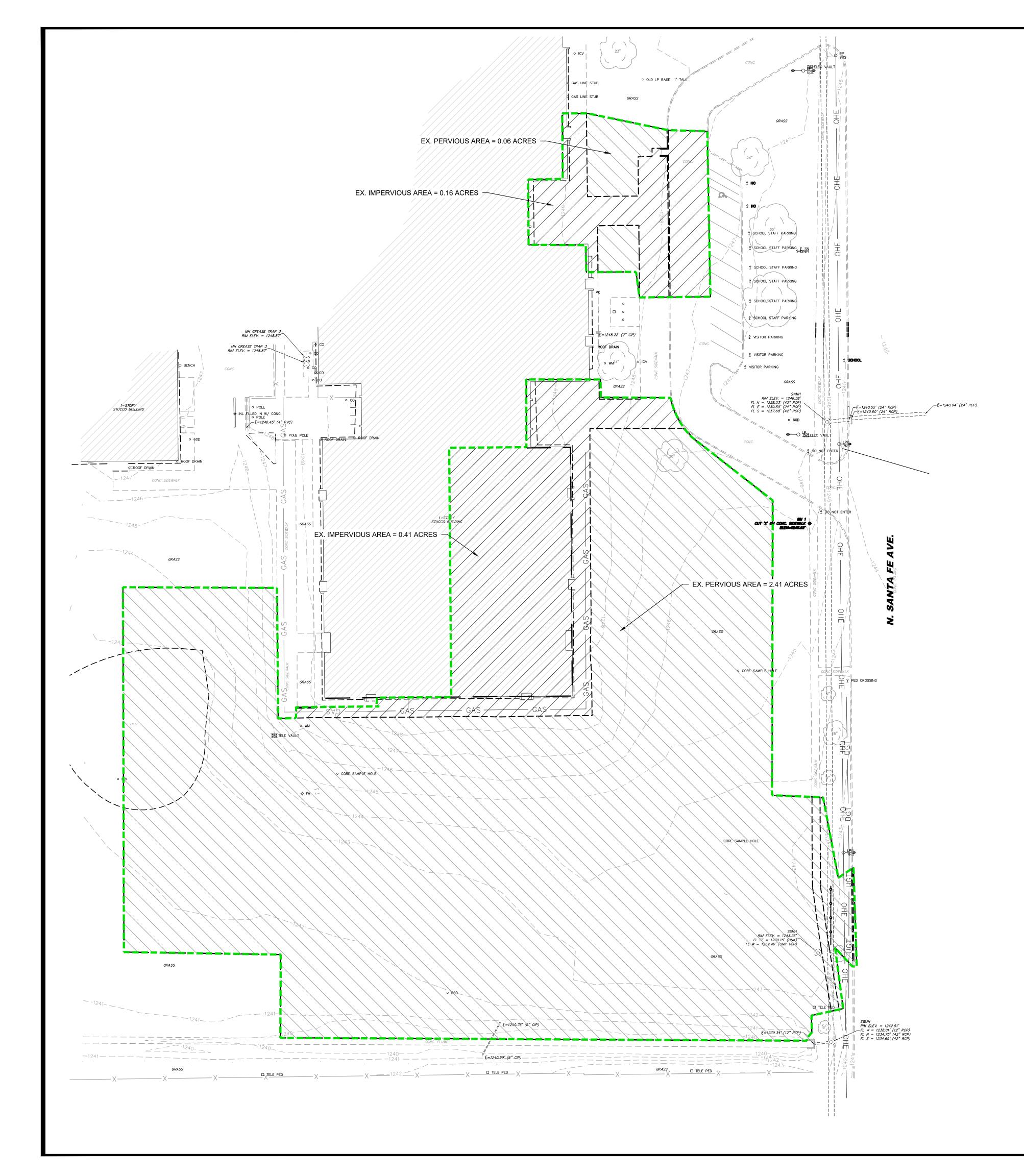
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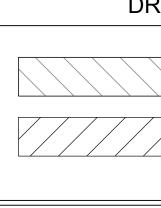


Legend

97°31'5"W 35°20'54"N SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT Without Base Flood Elevation (BFE) Zone A. V. A9 With BFE or Depth Zone AE, AO, AH, VE, AR SPECIAL FLOOD HAZARD AREAS **Regulatory Floodway** 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X Future Conditions 1% Annual Chance Flood Hazard Zone X Area with Reduced Flood Risk due to Levee. See Notes. Zone X OTHER AREAS OF FLOOD HAZARD Area with Flood Risk due to Levee Zone D NO SCREEN Area of Minimal Flood Hazard Zone X Effective LOMRs OTHER AREAS Area of Undetermined Flood Hazard Zone D - — – – Channel, Culvert, or Storm Sewer GENERAL STRUCTURES LITITIE Levee, Dike, or Floodwall City of Moore 20.2 Cross Sections with 1% Annual Chance 400044 17.5 Water Surface Elevation AREA OF MINIMAL FLOOD HAZARD **Coastal Transect** Base Flood Elevation Line (BFE) Limit of Study T10N R3W S16 Jurisdiction Boundary **Coastal Transect Baseline** ----T10N R3W S15 OTHER Profile Baseline 40027C0160J FEATURES Hydrographic Feature **Digital Data Available** No Digital Data Available MAP PANELS Unmapped The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location. This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map City of Oklahoma City was exported on 9/25/2023 at 3:18 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time. This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for 97°30'27"W 35°20'25"N Feet 1:6,000 unmapped and unmodernized areas cannot be used for regulatory purposes.

Basemap Imagery Source: USGS National Map 2023



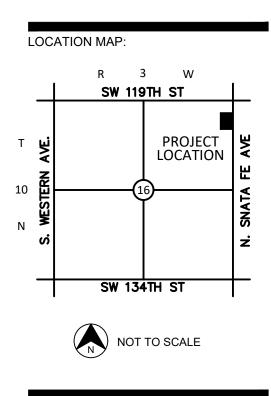


DRAINAGE LEGEND

EXISTING PERVIOUS CN = 80

EXISTING IMPERVIOUS CN = 98





PROJECT:

HIGHLAND WEST JR. HIGH

901 N. SANTA FE MOORE, OK

PROJECT NUMBER: 11.02.23 11.02.23 DRAWING DATE: ISSUE DATE:

SEAL:

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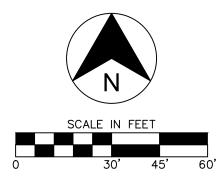
REVISIONS: <u> 11.02.23</u>

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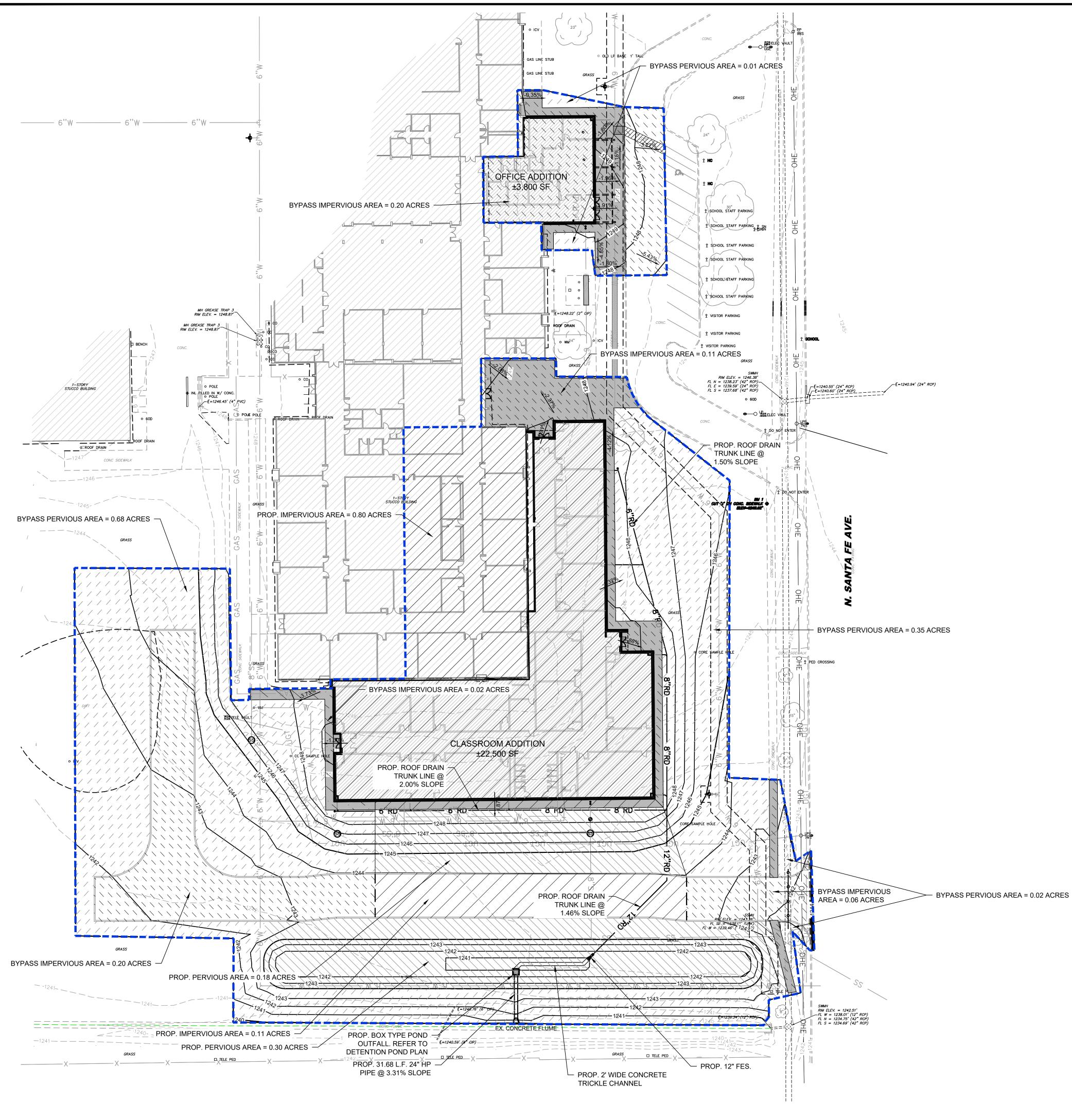


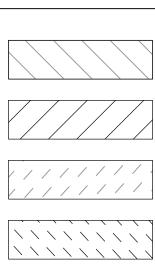


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DRAINAGE LEGEND

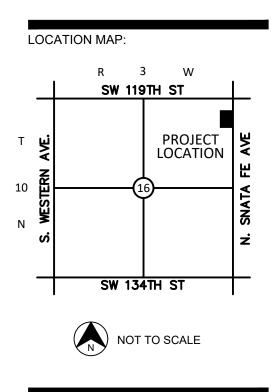
PROPOSED PERVIOUS CN = 80

PROPOSED IMPERVIOUS CN = 98

BYPASS IMPERVIOUS CN = 80

BYPASS IMPERVIOUS CN = 98





PROJECT:

HIGHLAND WEST JR. HIGH

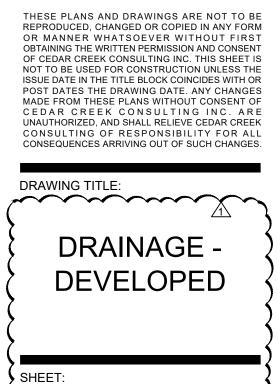
901 N. SANTA FE
MOORE, OK

PROJECT NUMBER:	23069
DRAWING DATE:	11.02.23
ISSUE DATE:	11.02.23

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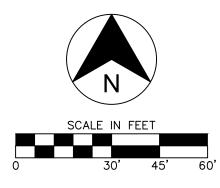
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REVISIONS: <u> 11.02.23</u> <u>CB #1</u>

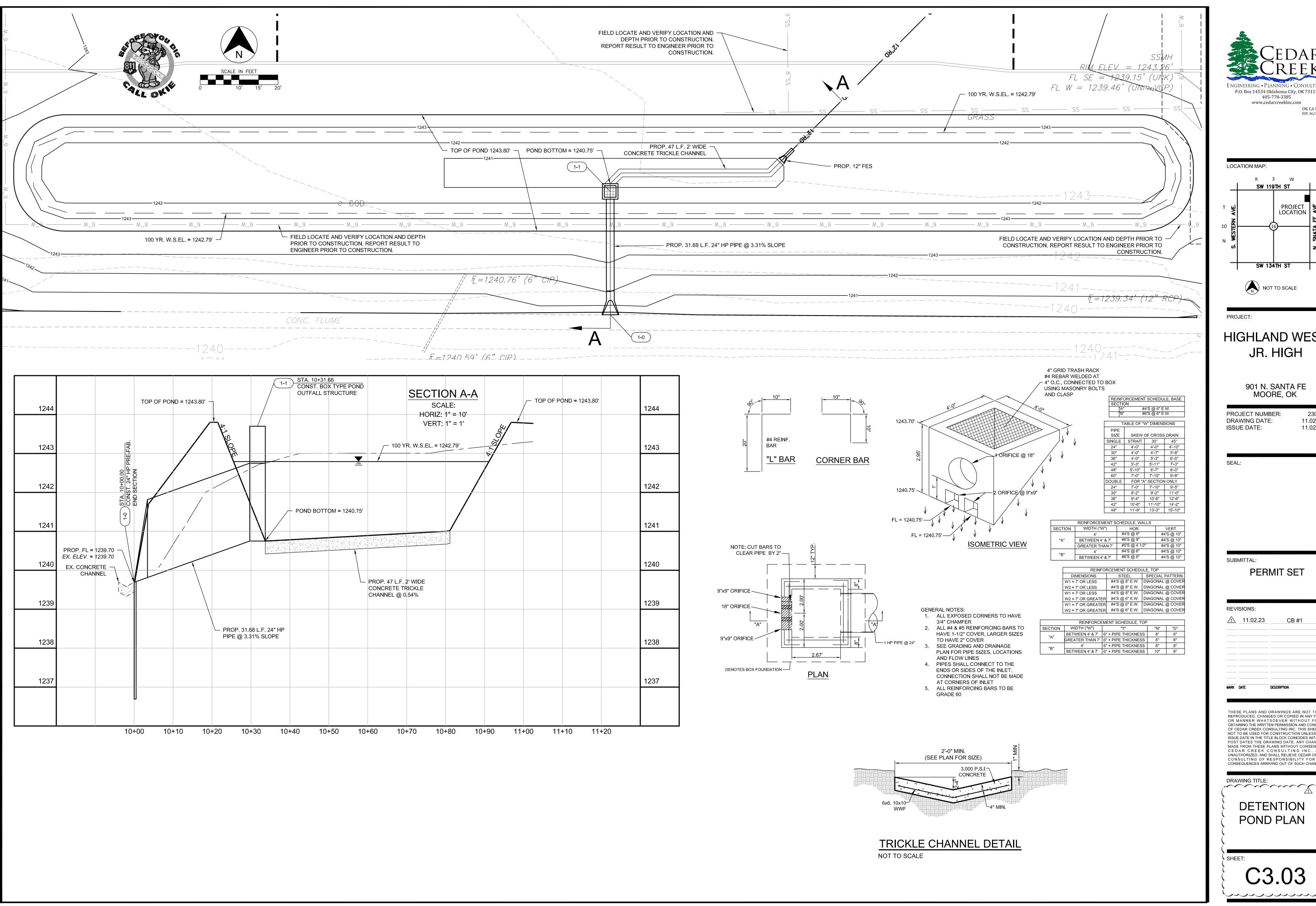


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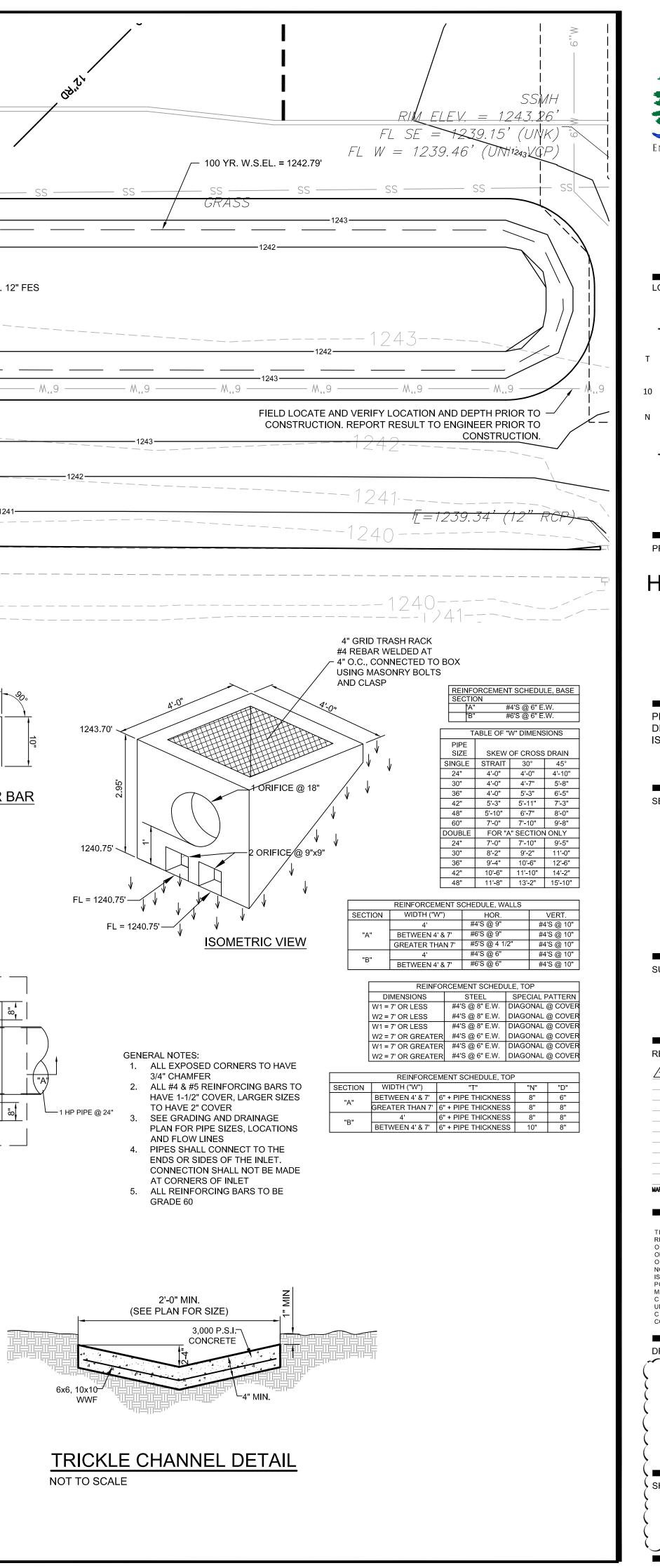
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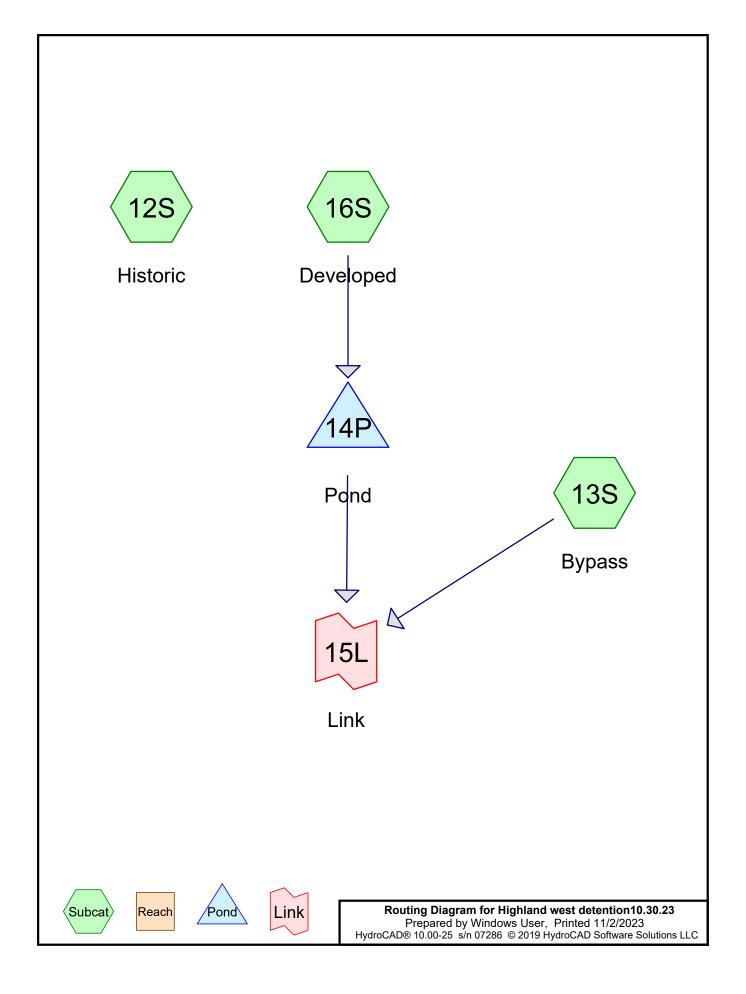




TION A-A SCALE: RIZ: 1" = 10'	/		DND = 1243.80'		1244
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	P.O. Box 14534 Oklahoma City, OK 73113 405-778-3385 www.cedarcreekinc.com OK CA 5864 EXP. 06/30/24
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DETENTION POND PLAN SHEET: C3.03	



Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
4.010	80	>75% Grass cover, Good, HSG D (12S, 13S, 16S)
2.070	98	Paved parking, HSG D (12S, 13S, 16S)

Highland west detention10.30.23 Prepared by Windows User HydroCAD® 10.00-25 s/n 07286 © 2019 Hydr	Type II 24-hr 2 year Rainfall=3.48"Printed 11/2/2023roCAD Software Solutions LLCPage 3
Runoff by SCS TF	0-20.00 hrs, dt=0.02 hrs, 751 points R-20 method, UH=SCS, Weighted-CN rans method - Pond routing by Stor-Ind method
Subcatchment12S: Historic	Runoff Area=3.040 ac 18.75% Impervious Runoff Depth>1.70" Tc=5.0 min CN=83 Runoff=10.31 cfs 0.431 af
Subcatchment13S: Bypass	Runoff Area=1.650 ac 35.76% Impervious Runoff Depth>1.93" Tc=5.0 min CN=86 Runoff=6.24 cfs 0.266 af
Subcatchment16S: Developed	Runoff Area=1.390 ac 65.47% Impervious Runoff Depth>2.45" Tc=5.0 min CN=92 Runoff=6.31 cfs 0.284 af
Pond 14P: Pond	Peak Elev=1,241.76' Storage=1,694 cf Inflow=6.31 cfs 0.284 af Outflow=4.27 cfs 0.284 af
Link 15L: Link	Inflow=10.22 cfs 0.549 af Primary=10.22 cfs 0.549 af

Summary for Subcatchment 12S: Historic

Runoff = 10.31 cfs @ 11.96 hrs, Volume= 0.431 af, Depth> 1.70"

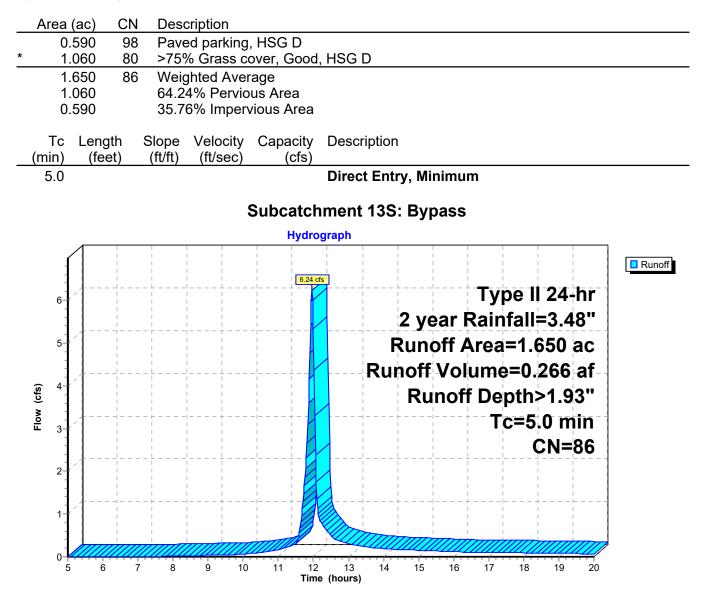
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 2 year Rainfall=3.48"

Area			cription										
	.570 98 .470 80		ed parking % Grass o	, HSG D over, Gooc		П							
3. 2.	.040 83 .470 .570	3 Weię 81.2	ghted Aver 5% Pervio	rage	<u>, 1100</u>	<u> </u>							
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Summary for Subcatchment 13S: Bypass

Runoff = 6.24 cfs @ 11.96 hrs, Volume= 0.266 af, Depth> 1.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 2 year Rainfall=3.48"



Summary for Subcatchment 16S: Developed

Runoff = 6.31 cfs @ 11.96 hrs, Volume= 0.284 af, Depth> 2.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 2 year Rainfall=3.48"

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Summary for Pond 14P: Pond

Inflow Area =	1.390 ac, 65.47% Impervious, Inflow Depth > 2.45" for 2 year event	
Inflow =	6.31 cfs @ 11.96 hrs, Volume= 0.284 af	
Outflow =	4.27 cfs @ 12.02 hrs, Volume= 0.284 af, Atten= 32%, Lag= 3.8 min	
Primary =	4.27 cfs @ 12.02 hrs, Volume= 0.284 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Peak Elev= 1,241.76' @ 12.02 hrs Surf.Area= 3,548 sf Storage= 1,694 cf

Plug-Flow detention time= 4.4 min calculated for 0.284 af (100% of inflow) Center-of-Mass det. time= 3.9 min (759.9 - 756.0)

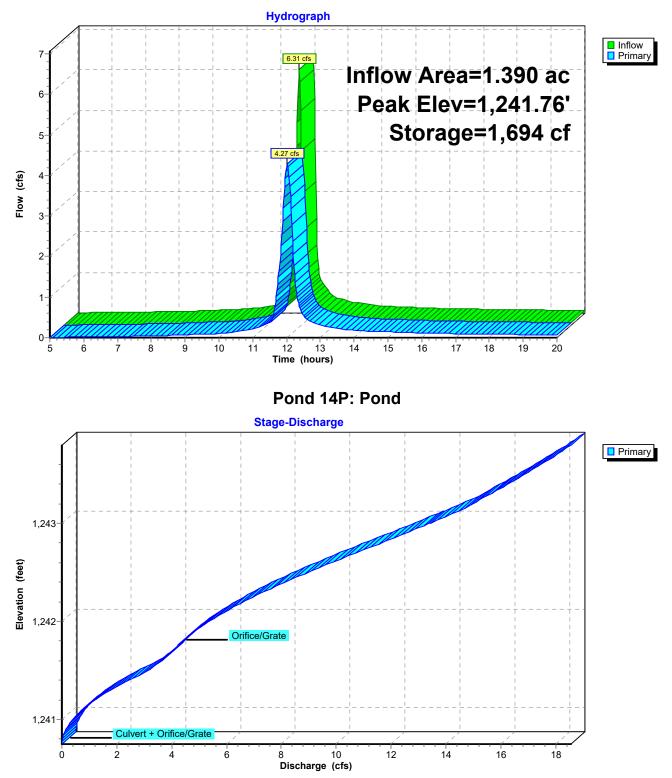
Volume	Inve	ert Avail.St	orage Storag	ge Description	
#1	1,240.7	75' 14,5	599 cf Custo	om Stage Data (P	rismatic)Listed below (Recalc)
-		0 ()			
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
1,240.7	75	0	0	0	
1,241.0	00	664	83	83	
1,242.0	00	4,436	2,550	2,633	
1,243.0	00	6,878	5,657	8,290	
1,243.3	35	7,756	2,561	10,851	
1,243.8	30	8,903	3,748	14,599	
Device	Routing	Inver	t Outlet Devie	ces	
#1	Primary	1,240.75	24.0" Roui	nd Culvert	
			L= 31.7' R	CP. square edge	headwall, Ke= 0.500
					'/ 1,239.70' S= 0.0331 '/' Cc= 0.900
				Flow Area= 3.14 s	
#2	Device 1	1.240.75			e/Grate X 2.00 C= 0.600
#3	Device 1	,		Orifice/Grate C	
	201100	., 0			

Primary OutFlow Max=4.27 cfs @ 12.02 hrs HW=1,241.76' TW=1,240.34' (Fixed TW Elev= 1,240.34') **1=Culvert** (Passes 4.27 cfs of 5.48 cfs potential flow)

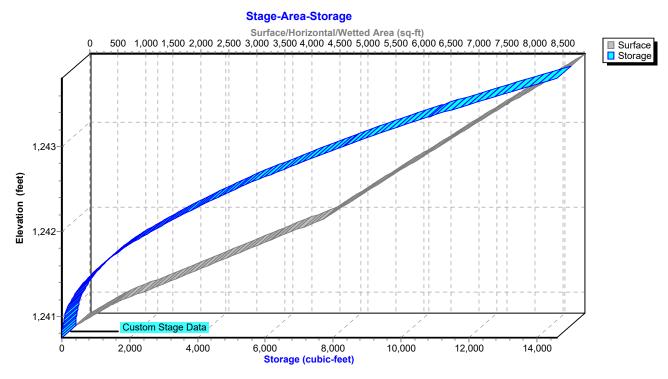
2=Orifice/Grate (Orifice Controls 4.26 cfs @ 3.79 fps)

-3=Orifice/Grate (Orifice Controls 0.00 cfs @ 0.41 fps)

Pond 14P: Pond



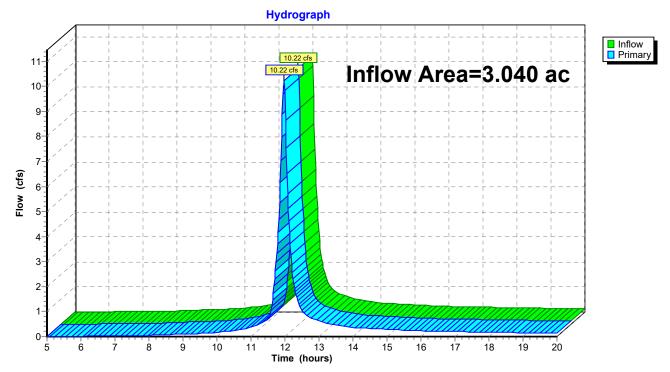
Pond 14P: Pond



Summary for Link 15L: Link

Inflow Area	a =	3.040 ac, 49.34% Impervious, Inflow Depth > 2.17" for 2 year event	
Inflow	=	10.22 cfs @ 11.97 hrs, Volume= 0.549 af	
Primary	=	10.22 cfs @ 11.97 hrs, Volume= 0.549 af, Atten= 0%, Lag= 0.0 mi	in

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs



Link 15L: Link

Highland west detention10.30.23 Prepared by Windows User HydroCAD® 10.00-25 s/n 07286 © 2019 Hydr	Type II 24-hr 5 year Rainfall=4.47" Printed 11/2/2023 roCAD Software Solutions LLC Page 11
Runoff by SCS TF	0-20.00 hrs, dt=0.02 hrs, 751 points R-20 method, UH=SCS, Weighted-CN rans method - Pond routing by Stor-Ind method
Subcatchment12S: Historic	Runoff Area=3.040 ac 18.75% Impervious Runoff Depth>2.51" Tc=5.0 min CN=83 Runoff=14.90 cfs 0.635 af
Subcatchment13S: Bypass	Runoff Area=1.650 ac 35.76% Impervious Runoff Depth>2.78" Tc=5.0 min CN=86 Runoff=8.77 cfs 0.382 af
Subcatchment16S: Developed	Runoff Area=1.390 ac 65.47% Impervious Runoff Depth>3.35" Tc=5.0 min CN=92 Runoff=8.43 cfs 0.388 af
Pond 14P: Pond	Peak Elev=1,241.99' Storage=2,591 cf Inflow=8.43 cfs 0.388 af Outflow=5.31 cfs 0.388 af
Link 15L: Link	Inflow=13.52 cfs 0.769 af Primary=13.52 cfs 0.769 af

Summary for Subcatchment 12S: Historic

Runoff = 14.90 cfs @ 11.96 hrs, Volume= 0.635 af, Depth> 2.51"

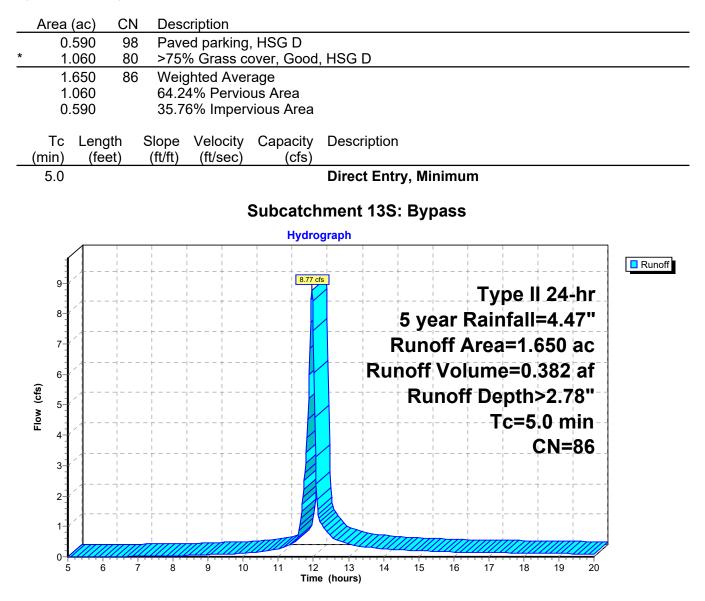
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 5 year Rainfall=4.47"

Area 0	<u>(ac) CN</u> .570 98		cription ed parking								
	.370 90 .470 80			over, Good	HSG E)					
2.	.040 83 .470 .570	3 Weię 81.2	ghted Aver 5% Pervio	age							
Tc min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Descri	ption					
5.0					Direct	Entry, M	linimu	m			
				Subcatch	mont	126. 11	storio				
			,			123. ПВ	SUTIC				
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11-1	/	+	+		!		1		1	040 ac	-
10						Runof	f Vol	ume)=0 .	635 af	_
9	/ /	+	 +			RI	unof	f De	pth	>2.51"	_
100 (cla)	/	<u> </u>					 	T	c=5	.0 min	_
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Summary for Subcatchment 13S: Bypass

Runoff = 8.77 cfs @ 11.96 hrs, Volume= 0.382 af, Depth> 2.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 5 year Rainfall=4.47"



Summary for Subcatchment 16S: Developed

Runoff = 8.43 cfs @ 11.96 hrs, Volume= 0.388 af, Depth> 3.35"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 5 year Rainfall=4.47"

Ar	ea	(a	c)	CN		criptio												
		.9		98	Pave	ed par	king,	HSG	D		`							
		.48 .39		80 92		% Gra ghted /		over, G	000,	HSGL)							
		.38 .48		92				us Are	а									
	0	.9	10		65.4	7% In	nperv	ious A	rea									
- (mi	Гc n)	L	engtl. (feet		Slope (ft/ft)	Velo (ft/s		Capa (c	city cfs)	Descr	iption	I						
· ·	.0			/						Direct	t Ent	ry, Mi	nimu	m				
							-											
							Su	bcat	chm	ent 1	6S:	Deve	lope	d				
								H	lydrog	raph								
	. 1																	Runof
	9-			 					8.43 cfs)				-		104		
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	_			i 1				+				5 ye	ear-F	Rair	nfall	=4.4	7"	
	7-			1					F		F	Rund	off A	Area	a=1.	390	ac	
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fs)	5-			 	- 	 		 + 								>3.3		
Flow (cfs)	9- -			1								nu			1	1 1		
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	-			 		 								L		CN=	92	
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	5)	6	7	8	9	10	11	12 Time	13 (hours)	14	15	16	17	18	19	20	

Summary for Pond 14P: Pond

Inflow Area =	1.390 ac, 65.47% Impervious, Inflow Depth > 3.35" for 5 year event	
Inflow =	8.43 cfs @ 11.96 hrs, Volume= 0.388 af	
Outflow =	5.31 cfs @ 12.03 hrs, Volume= 0.388 af, Atten= 37%, Lag= 4.1 min	
Primary =	5.31 cfs @ 12.03 hrs, Volume= 0.388 af	

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Peak Elev= 1,241.99' @ 12.03 hrs Surf.Area= 4,400 sf Storage= 2,591 cf

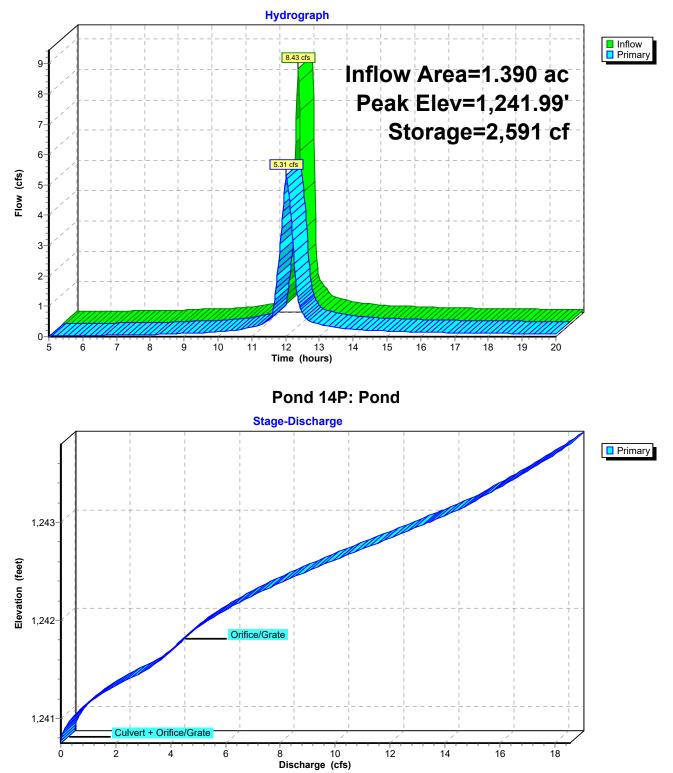
Plug-Flow detention time= 4.9 min calculated for 0.388 af (100% of inflow) Center-of-Mass det. time= 4.4 min (754.1 - 749.7)

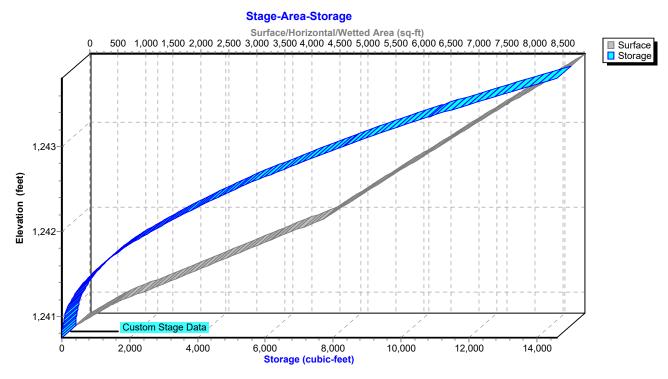
Volume	Inve	ert Avail.St	orage Ste	orage	Description		
#1	1,240.7	75' 14,5	599 cf Cı	stom	Stage Data (P	rismatic)Listed below (Re	ecalc)
F lavestia					Ourse Otherse		
Elevatio		Surf.Area	Inc.Sto		Cum.Store		
(fee	t)	(sq-ft)	(cubic-fe	et)	(cubic-feet)		
1,240.7	5	0		0	0		
1,241.0	0	664		83	83		
1,242.0	0	4,436	2,5	50	2,633		
1,243.0	0	6,878	5,6	57	8,290		
1,243.3	5	7,756	2,5	61	10,851		
1,243.8	0	8,903	3,7	48	14,599		
Device	Routing	Inver	: Outlet D	evice	S		
#1	Primary	1,240.75	' 24.0" F	ound	l Culvert		
	-		L= 31.7	RC	P. square edge	headwall, Ke= 0.500	
						5' / 1,239.70' S= 0.0331 '/	' Cc= 0.900
					w Area= 3.14 s		
#2	Device 1	1.240.75		,		Grate X 2.00 C= 0.600	
#3	Device 1	,			rifice/Grate C		
110	201100	.,2					
		-					

Primary OutFlow Max=5.29 cfs @ 12.03 hrs HW=1,241.99' TW=1,240.34' (Fixed TW Elev= 1,240.34') **1=Culvert** (Passes 5.29 cfs of 7.74 cfs potential flow)

2=Orifice/Grate (Orifice Controls 4.99 cfs @ 4.44 fps)

-3=Orifice/Grate (Orifice Controls 0.30 cfs @ 1.66 fps)

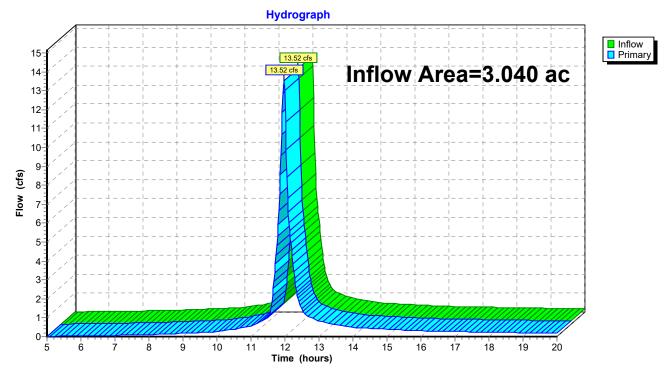




Summary for Link 15L: Link

Inflow Are	a =	3.040 ac, 49.34% Impervious, Inflow Depth > 3.04" for 5 year event	
Inflow	=	13.52 cfs @ 11.97 hrs, Volume= 0.769 af	
Primary	=	13.52 cfs @ 11.97 hrs, Volume= 0.769 af, Atten= 0%, Lag= 0.0 n	nin

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs



Link 15L: Link

Highland west detention10.30.23 Prepared by Windows User HydroCAD® 10.00-25 s/n 07286 © 2019 Hyd	· · · · ·
Runoff by SCS T	00-20.00 hrs, dt=0.02 hrs, 751 points R-20 method, UH=SCS, Weighted-CN Trans method - Pond routing by Stor-Ind method
Subcatchment12S: Historic	Runoff Area=3.040 ac 18.75% Impervious Runoff Depth>3.27" Tc=5.0 min CN=83 Runoff=19.13 cfs 0.829 af
Subcatchment13S: Bypass	Runoff Area=1.650 ac 35.76% Impervious Runoff Depth>3.57" Tc=5.0 min CN=86 Runoff=11.08 cfs 0.491 af
Subcatchment16S: Developed	Runoff Area=1.390 ac 65.47% Impervious Runoff Depth>4.17" Tc=5.0 min CN=92 Runoff=10.34 cfs 0.483 af
Pond 14P: Pond	Peak Elev=1,242.17' Storage=3,425 cf Inflow=10.34 cfs 0.483 af Outflow=6.40 cfs 0.483 af
Link 15L: Link	Inflow=16.68 cfs 0.973 af Primary=16.68 cfs 0.973 af

Summary for Subcatchment 12S: Historic

Runoff = 19.13 cfs @ 11.96 hrs, Volume= 0.829 af, Depth> 3.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 10 year Rainfall=5.37"

Area	(ac) CN	N Des	cription				
	.570 98		ed parking				
	.470 80			over, Good	SG D		
	.040 83		ghted Ave				
	.470	-	5% Pervio				
0	.570	18.7	5% Imper	vious Area			
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·		
5.0					Direct Entry, Minimum		
				Subcatch	ent 12S: Historic		
				Hydro	iph		
	1				·		
21 - 20 -	/		! 	_ - 19.13 (· ¹ /		Runof
19		+				e II 24-hr	
18	/	 	 +				
17∃ŕ 16∃ŕ	/	<u>+</u>			10 year Rainf		
15		 	 		Runoff Area=	=3.040 ac	
14		+	 +		Runoff Volume	=0 829 af	
13 (2) 12		+	+ ·	-			
LIOM (cfs)			 		Runoff Dep)tn>3.27"	
8 10		<u> </u>	 		·¦¦¦ T c	=5.0 min	
■ 9 8		+	 		· +	CN=83	
7		+	 +		· +	CIN-0 5	
6		<u>1</u>			·		
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1 -1 0							
5	6	7 8	9 10	11 12	13 14 15 16 17	18 19 20	

Summary for Subcatchment 13S: Bypass

Runoff = 11.08 cfs @ 11.96 hrs, Volume= 0.491 af, Depth> 3.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 10 year Rainfall=5.37"

Area	(ac) CN	I Dese	cription										
	590 98	B Pave	ed parking	, HSG D		-							
	060 80			over, Good	, HSG	D							
	.650 86 .060		ghted Aver 4% Pervio										
	590			/ious Area									
0.		00.1	o /o import	100071100									
Тс	Length	Slope	Velocity	Capacity	Desci	iption	1 I						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)									
5.0					Direc	t Ent	ry, Miı	nimur	n				
				Cubaatak		420							
			÷	Subcatch		135	: вур	Dass					
				Hydro	graph								
12-		- +			+	+			+		+		Runoff
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11-1	ļ		 	·	1 !	ا بــــــ	 	 	ly	pe II	24-	hr	
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- 8	/		 			1							
•	/	· - +		 	·	Rui					491		
Flow (cfs)	/	-+			·		Ru	noff	De	pth	>3.5	7"	
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5	6	7 8	9 10	11 12 Time	13 (hours)	14	15	16	17	18	19	20	

Summary for Subcatchment 16S: Developed

Runoff = 10.34 cfs @ 11.96 hrs, Volume= 0.483 af, Depth> 4.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 10 year Rainfall=5.37"

Area			cription							
	.910 98 .480 80	8 Pave	ed parking	, HSG D over, Good						
1. 0.	.390 92 .480 .910	2 Weię 34.5	ghted Aver 3% Pervio	rage	, 1130 D					
Tc min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Descri	ption				
5.0					Direct	Entry, Mi	inimur	n		
			9	ubcatchn	nont 16		مماه	Ч		
				Hydro		JO. Deve	siope	u		
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0	6	7 8	9 10	11 12	13	14 15	16	17 18	19 20	-

Summary for Pond 14P: Pond

Inflow Area =	1.390 ac, 65.47% Impervious, Ir	nflow Depth > 4.17" for 10 year event
Inflow =	10.34 cfs @ 11.96 hrs, Volume=	0.483 af
Outflow =	6.40 cfs @ 12.03 hrs, Volume=	0.483 af, Atten= 38%, Lag= 4.2 min
Primary =	6.40 cfs @ 12.03 hrs, Volume=	0.483 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Peak Elev= 1,242.17' @ 12.03 hrs Surf.Area= 4,852 sf Storage= 3,425 cf

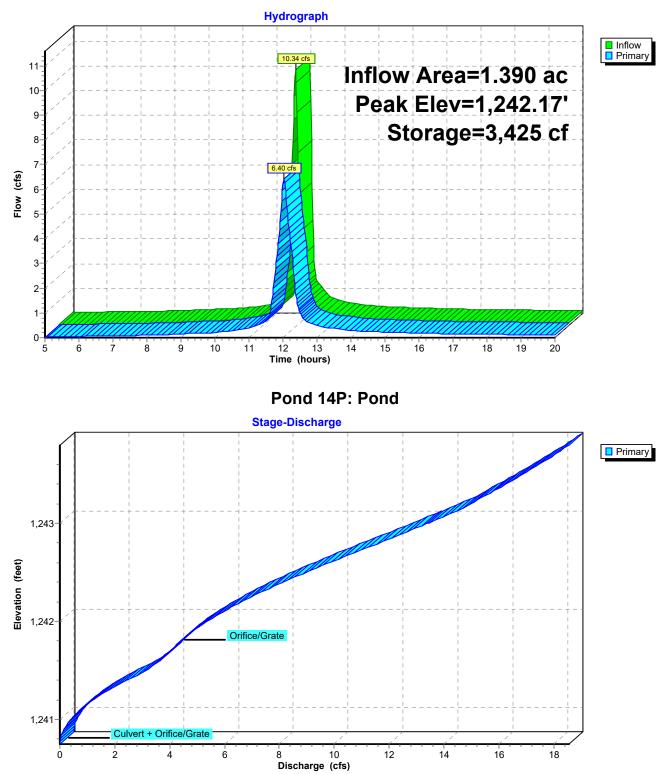
Plug-Flow detention time= 5.2 min calculated for 0.483 af (100% of inflow) Center-of-Mass det. time= 4.8 min (750.5 - 745.7)

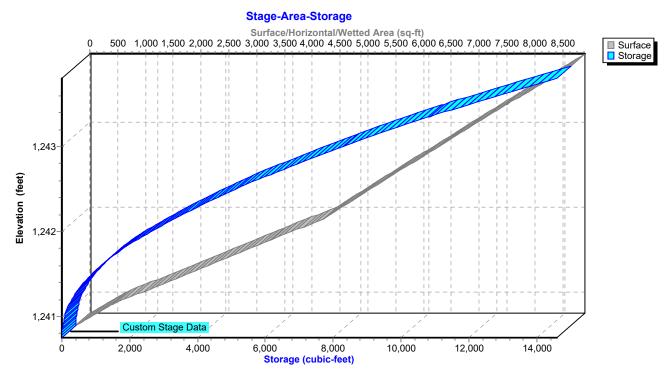
Volume	Inv	ert Avail.St	orage Storag	e Description	
#1	1,240.	75' 14,5	599 cf Custo	m Stage Data (P	rismatic)Listed below (Recalc)
F 1				0	
Elevatio		Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
1,240.7	75	0	0	0	
1,241.0	00	664	83	83	
1,242.0	00	4,436	2,550	2,633	
1,243.0	00	6,878	5,657	8,290	
1,243.3	35	7,756	2,561	10,851	
1,243.8	30	8,903	3,748	14,599	
Device	Routing	Inver	Outlet Device	ces	
#1	Primary	1,240.75	24.0" Rour	nd Culvert	
	-		L= 31.7' R	CP. square edge	headwall, Ke= 0.500
					'/1,239.70' S=0.0331 '/' Cc=0.900
				low Area= 3.14 s	
#2	Device 1	1.240.75			/Grate X 2.00 C= 0.600
#3	Device 1	,		Orifice/Grate C	
	2 2 7 100	.,			

Primary OutFlow Max=6.38 cfs @ 12.03 hrs HW=1,242.17' TW=1,240.34' (Fixed TW Elev= 1,240.34') **1=Culvert** (Passes 6.38 cfs of 9.65 cfs potential flow)

2=Orifice/Grate (Orifice Controls 5.50 cfs @ 4.89 fps)

-3=Orifice/Grate (Orifice Controls 0.88 cfs @ 2.20 fps)

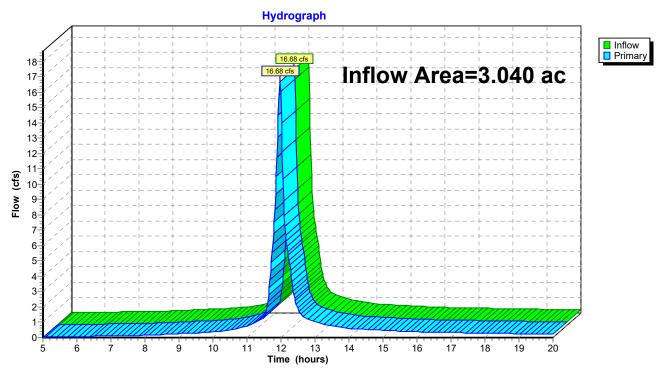




Summary for Link 15L: Link

Inflow Are	a =	3.040 ac, 49.34% Impervious, Inflow Depth > 3.84" for 10 year event	
Inflow	=	16.68 cfs @ 11.97 hrs, Volume= 0.973 af	
Primary	=	16.68 cfs @ 11.97 hrs, Volume= 0.973 af, Atten= 0%, Lag= 0.0 m	nin

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs



Link 15L: Link

Highland west detention10.30.23 Prepared by Windows User HydroCAD® 10.00-25 s/n 07286 © 2019 Hyd	<i>Type II 24-hr 50 year Rainfall=7.94"</i> Printed 11/2/2023 droCAD Software Solutions LLC Page 27
Runoff by SCS 1	00-20.00 hrs, dt=0.02 hrs, 751 points FR-20 method, UH=SCS, Weighted-CN Trans method - Pond routing by Stor-Ind method
Subcatchment12S: Historic	Runoff Area=3.040 ac 18.75% Impervious Runoff Depth>5.55" Tc=5.0 min CN=83 Runoff=31.28 cfs 1.405 af
Subcatchment13S: Bypass	Runoff Area=1.650 ac 35.76% Impervious Runoff Depth>5.88" Tc=5.0 min CN=86 Runoff=17.65 cfs 0.809 af
Subcatchment16S: Developed	Runoff Area=1.390 ac 65.47% Impervious Runoff Depth>6.52" Tc=5.0 min CN=92 Runoff=15.76 cfs 0.755 af
Pond 14P: Pond	Peak Elev=1,242.60' Storage=5,757 cf Inflow=15.76 cfs 0.755 af Outflow=9.84 cfs 0.754 af
Link 15L: Link	Inflow=26.13 cfs 1.563 af Primary=26.13 cfs 1.563 af

Summary for Subcatchment 12S: Historic

Runoff = 31.28 cfs @ 11.96 hrs, Volume= 1.405 af, Depth> 5.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 50 year Rainfall=7.94"

Area	(ac) CN	Des	cription										
	.570 98		ed parking			-							
	.470 80			over, Good	, HSG	D							
	.040 83		ghted Ave										
	.470 .570	-	5% Pervio	vious Area									
0	.570	10.7	5% imper	NOUS AIEa									
Тс	Length	Slope	Velocity	Capacity	Desc	ription							
min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		•							
5.0					Direc	t Entr	Ƴ, Mir	nimur	n				
			ę	Subcatch	nment	12S:	Hist	oric					
				Hydro	ograph								
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28						5	0 ye	ar F	Rain	fall:	=7 9	4 "	
26		+				I I	- 1						
24	/	<u>+</u>	 			! +	Runc		<u>_</u>	!			
22	/	<u> </u>	 		/	Rur	າoff	Vol	ume	e=1.	405	af	
20-	}	+	 				Ru	noff	De	oth	>5.5	5"	
18 18 16	/	+	 +		/	 + 			+				
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12			' 		/	' 	L	 	^L 	Q	CN=	83	
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6		+	 		/	 +				!			
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2													
0 1 5	6	7 8	9 10		13	-/ 14	15	16	/ 17	18	19	20	

Summary for Subcatchment 13S: Bypass

Runoff = 17.65 cfs @ 11.96 hrs, Volume= 0.809 af, Depth> 5.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 50 year Rainfall=7.94"

	(ac) Cl .590 9		cription ed parking		
	.060 8			over, Good	d, HSG D
1	.650 8 .060 .590	6 Wei 64.2	ghted Aver 4% Pervio	age	
Tc min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	
5.0					Direct Entry, Minimum
				Subaatab	abment 128, Bynese
			•		chment 13S: Bypass
		1		Hydro	rograph
19		<u>+</u>		- 	
18		<u>↓</u>	 	17.65 cl	
17		+	 		Type II 24-hr
16 - 15-	·	+	+		50 year Rainfall=7.94"
14		+ +	+ 		Runoff Area=1.650 ac
13	/	+	 +		Runoff Volume=0.809 af
12-	//	+	+ 		
11 10 9		+ ! +	+ 		Runoff Depth>5.88"
9	()	+	, +		Tc=5.0 min
8		+	 +		CN=86
7- 6-		T I		- i	
5		+ +	+ +		
4	/		+		·····
3	/	 	 		
2- 1-		'	;;		
0		<u></u>	· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·	
5	6	7 8	9 10	11 12 Time	12 13 14 15 16 17 18 19 20 me (hours)

Summary for Subcatchment 16S: Developed

Runoff = 15.76 cfs @ 11.96 hrs, Volume= 0.755 af, Depth> 6.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 50 year Rainfall=7.94"

	.910 98 .480 80	B Pave	ed parking	, HSG D over, Good		D				
1. 0.	.390 92 .480 .910	2 Weią 34.5	ghted Aver 3% Pervio	age	<u>, пос</u>	U				
Tc min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)		ription				
5.0					Direc	t Entry, N	linimu	m		
			S	ubcatchn	nent 1	6S: Dev	elope	d		
				Hydro	graph		-			
17 16										
15		-							24-hr	_
14-7 13-7	/	+ +	+		/	· · · ·	1 1		=7.94"	-
12		<u>1</u>							390 ac	-
11 10- 10-	/						1 1		.755 af	-
9 8 8	/ /	+	 +			K	unon	f Depth	+	-
2 8 - 7	/	<u> </u> 						1	5.0 min	-
6	/		 					$ \frac{1}{1}$ $ -$	CN=92	_
5-1 4-1	/	+								-
3		+ +	+			+ +		+ 	+	-
2-* 1-*	/							<u>i</u>		-
	///////////////////////////////////////	///////////////////////////////////////								7

Summary for Pond 14P: Pond

Inflow Area	=	1.390 ac, 65.47% Impervious, Inflow Depth > 6.52" for 50 year event
Inflow =	=	15.76 cfs @ 11.96 hrs, Volume= 0.755 af
Outflow =	=	9.84 cfs @ 12.03 hrs, Volume= 0.754 af, Atten= 38%, Lag= 4.2 min
Primary =	=	9.84 cfs @ 12.03 hrs, Volume= 0.754 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Peak Elev= 1,242.60' @ 12.03 hrs Surf.Area= 5,911 sf Storage= 5,757 cf

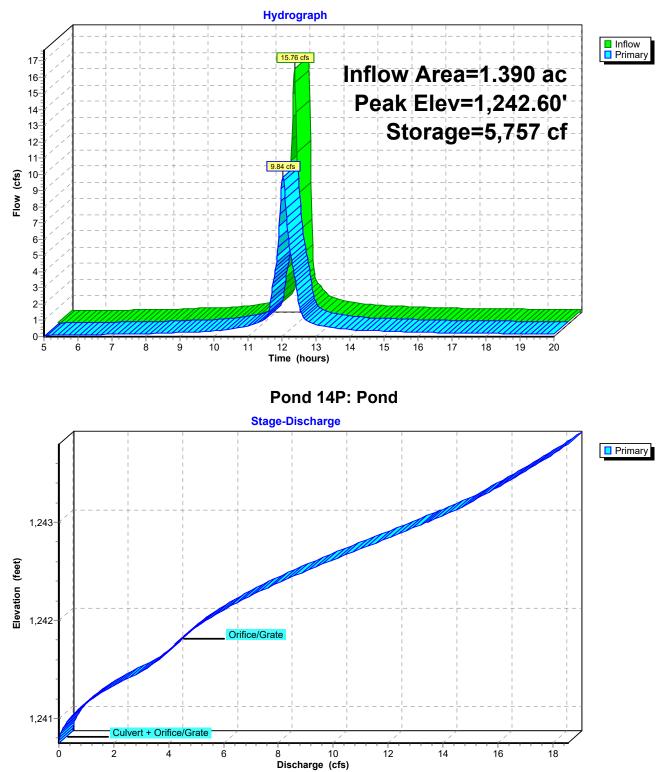
Plug-Flow detention time= 5.9 min calculated for 0.754 af (100% of inflow) Center-of-Mass det. time= 5.4 min (744.3 - 738.8)

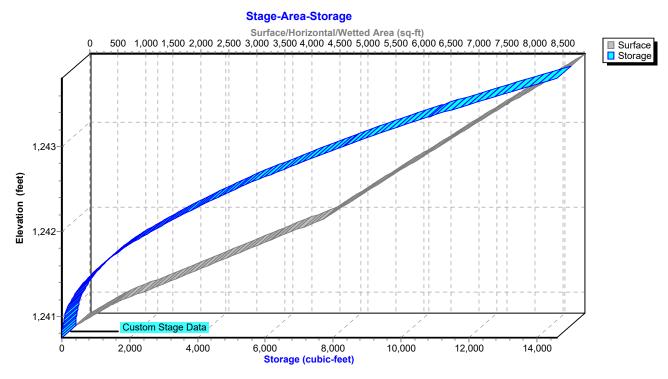
Volume	Inve	ert Avail.S	torage Storag	ge Description	
#1	1,240.7	75' 14,	599 cf Custo	om Stage Data (P	rismatic)Listed below (Recalc)
		o ()			
Elevatio	า	Surf.Area	Inc.Store	Cum.Store	
(feet)	(sq-ft)	(cubic-feet)	(cubic-feet)	
1,240.7	5	0	0	0	
1,241.0	0	664	83	83	
1,242.0	0	4,436	2,550	2,633	
1,243.0	0	6,878	5,657	8,290	
1,243.3	5	7,756	2,561	10,851	
1,243.8	0	8,903	3,748	14,599	
Device	Routing	Inver	t Outlet Devi	ces	
#1	Primary	1,240.75	5' 24.0" Rou	nd Culvert	
			L= 31.7' R	CP, square edge	headwall, Ke= 0.500
			Inlet / Outle	t Invert= 1,240.75	'/ 1,239.70' S= 0.0331 '/' Cc= 0.900
			n= 0.013, F	-low Area= 3.14 s	f
#2	Device 1	1,240.75			e/Grate X 2.00 C= 0.600
#3	Device 1	1,241.75	5' 18.0" Vert.	Orifice/Grate C	= 0.600
		,			

Primary OutFlow Max=9.81 cfs @ 12.03 hrs HW=1,242.60' TW=1,240.34' (Fixed TW Elev= 1,240.34') **1=Culvert** (Passes 9.81 cfs of 14.06 cfs potential flow)

2=Orifice/Grate (Orifice Controls 6.56 cfs @ 5.83 fps)

-3=Orifice/Grate (Orifice Controls 3.25 cfs @ 3.14 fps)

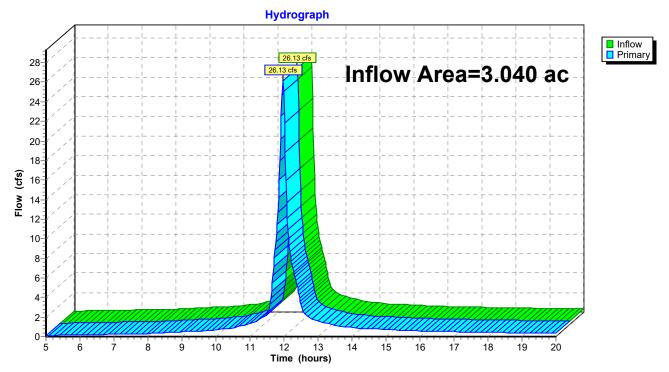




Summary for Link 15L: Link

Inflow Area	a =	3.040 ac, 49.34% Impervious, Inflow Depth > 6.17" for 50 year event	
Inflow	=	26.13 cfs @ 11.97 hrs, Volume= 1.563 af	
Primary	=	26.13 cfs @ 11.97 hrs, Volume= 1.563 af, Atten= 0%, Lag= 0.0 mir	า

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs



Link 15L: Link

	00-20.00 hrs, dt=0.02 hrs, 751 points
	FR-20 method, UH=SCS, Weighted-CN Trans method - Pond routing by Stor-Ind method
Subcatchment12S: Historic	Runoff Area=3.040 ac 18.75% Impervious Runoff Depth>6.73" Tc=5.0 min CN=83 Runoff=37.44 cfs 1.705 af
Subcatchment13S: Bypass	Runoff Area=1.650 ac 35.76% Impervious Runoff Depth>7.07" Tc=5.0 min CN=86 Runoff=20.96 cfs 0.973 af
Subcatchment16S: Developed	Runoff Area=1.390 ac 65.47% Impervious Runoff Depth>7.71" Tc=5.0 min CN=92 Runoff=18.49 cfs 0.893 af
Pond 14P: Pond	Peak Elev=1,242.79' Storage=6,931 cf Inflow=18.49 cfs 0.893 af Outflow=11.56 cfs 0.892 af
Link 15L: Link	Inflow=30.97 cfs 1.865 af Primary=30.97 cfs 1.865 af

Summary for Subcatchment 12S: Historic

Runoff = 37.44 cfs @ 11.96 hrs, Volume= 1.705 af, Depth> 6.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 100 year Rainfall=9.25"

Area	(ac)	CN	Dese	criptio	n											
	.570	98	Pave	ed par	king,	HSG	D									
	.470	80					Good	, HSG	D							
	.040	83		ghted												
	.470		-	5% Pe												
0	.570		18.7	5% In	nperv	ious A	Area									
Тс	Length	ו S	lope	Velo	city	Сара	icity	Desc	riptior	۱						
(min)	(feet) ((ft/ft)	(ft/s	ec)	(cfs)									
5.0								Direc	t Ent	ry, M	inimu	m				
					_	_	-				_					
					S	Subca	atch	ment	12S	: His	toric					
						l l	Hydro	graph								
4						 				 	 					
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38	()	+				, 	37.44 C	s 		↓ ↓	- 	Tv	no l	24	hr	
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34 - 32 -		+		+					10)0 y	ear l	Rain	fall	=9.2	5"	
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28	() 	 				 	¦			<u> </u>	<u> </u>					
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Summary for Subcatchment 13S: Bypass

Runoff = 20.96 cfs @ 11.96 hrs, Volume= 0.973 af, Depth> 7.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 100 year Rainfall=9.25"

А	rea	(ac)	CN E	Descr	ription											
	0.	590	98 F	Paveo	d parking	g, HSG	D									
		060			Grass		Good,	HSG	D							
		650			nted Ave											
		060 590	-		% Pervi % Impe											
	0.	590	C	55.70	70 imper	vious P	Nea									
	Тс	Length	n Slo	ре	Velocity	Capa	city	Desci	iptior	า						
(m	in)	(feet)		, /ft)	(ft/sec)	. (cfs)		-							
Ę	5.0							Direc	t Ent	ry, Mi	nimur	n				
						• • •			400							
						Subca	atch	ment	138	s: Byb	ass					
						ŀ	lydro	graph								
	23		· -	¦-	 		¦ !	+ +				 	·	 +		Runof
	22	/	· + -	-	+		20.96 cfs	 		 + -		+	 	+		
	21 - 20 -	/	· + -	;- !-	'			i				Ту	pe ll	24-	hr	
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_	14	/	+-	!-	+			!	Ru	noff	VOI	ume)= 0.	973	at	
Flow (cfs)	13 12		· + -	i-	<u>+</u>			:i		Ru	noff	De	pth>	>7.0	7"	
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	5	6	7	8	9 10) 11	12 Time	13 (hours)	14	15	16	17	18	19	20	

Summary for Subcatchment 16S: Developed

Runoff = 18.49 cfs @ 11.96 hrs, Volume= 0.893 af, Depth> 7.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Type II 24-hr 100 year Rainfall=9.25"

	.910 9	98 Pav	cription ed parking							
1. 0.		92 Wei 34.5	% Grass c ghted Ave 53% Pervic 17% Imper	rage ous Area						
Tc min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Descrip					
5.0					Direct	Entry, Mi	nimum			
			S	ubcatch	nent 16	S: Deve	loped			
				Hydr	ograph					_
20 19		+ + -			- +			+	 + +	Runo
19- 18- 17-		+ + 	++ +++ ++++++++++			+ ++ ++		-	l 24-hr	
16	/	-							=9.25"	
15 - 14-	 	± ±							390 ac	
13 - 12 -	/i	+ +			i-F				893 af	
CIS) 11 (CIS) 11 (CIS	/	+		-	·	¦-Ru	noff	Depth	>7.71"	
0 10 9 10	/	+ 	-1		¦	+ 		Tc=5	.0 min	
8- 7-		<u> </u>				<u> </u>			CN=92	
6	/ /	<u>+</u>					 	<u> </u>	 	
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1										ļ
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Summary for Pond 14P: Pond

Inflow Area =	1.390 ac, 65.47% Impervious, Inflo	w Depth > 7.71" for 100 year event
Inflow =	18.49 cfs @ 11.96 hrs, Volume=	0.893 af
Outflow =	11.56 cfs @ 12.03 hrs, Volume=	0.892 af, Atten= 37%, Lag= 4.2 min
Primary =	11.56 cfs @ 12.03 hrs, Volume=	0.892 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs Peak Elev= 1,242.79' @ 12.03 hrs Surf.Area= 6,377 sf Storage= 6,931 cf

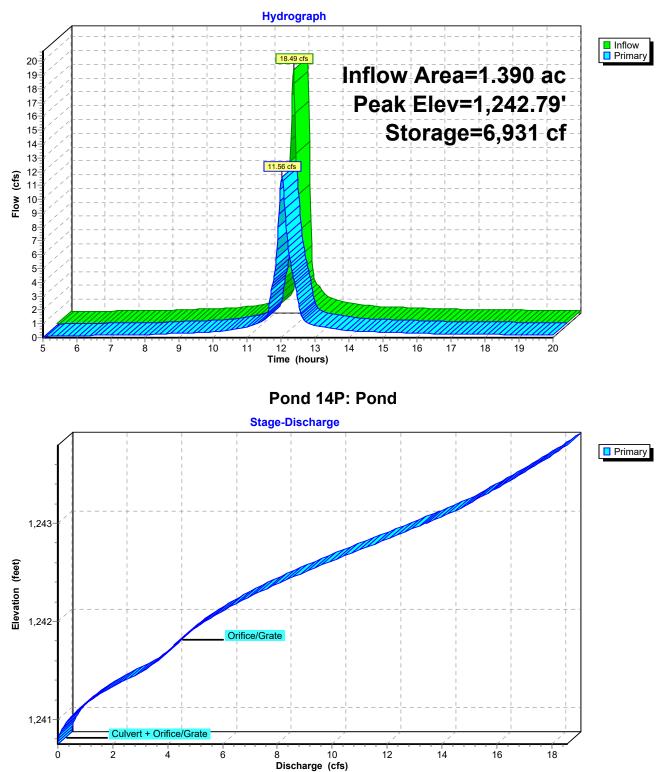
Plug-Flow detention time= 6.2 min calculated for 0.892 af (100% of inflow) Center-of-Mass det. time= 5.7 min (742.3 - 736.7)

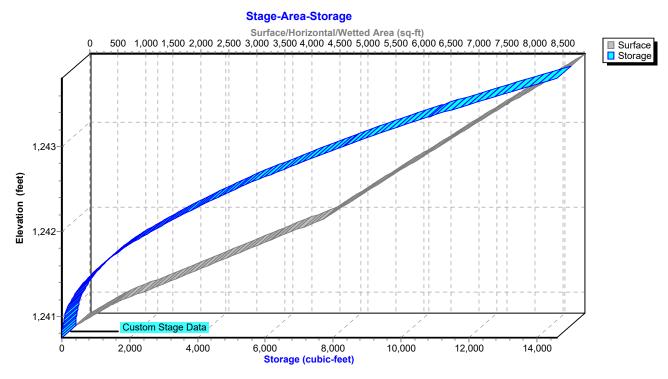
Volume	Inve	ert Avail.S	torage Storag	rage Storage Description		
#1	1,240.7	75' 14 ,	599 cf Custo	m Stage Data (P	rismatic)Listed below (Recalc)	
- 1				0		
Elevatio		Surf.Area	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)		
1,240.7	75	0	0	0		
1,241.0	00	664	83	83		
1,242.0	00	4,436	2,550	2,633		
1,243.0	00	6,878	5,657	8,290		
1,243.3	35	7,756	2,561	10,851		
1,243.8	30	8,903	3,748	14,599		
Device	Routing	Inver	t Outlet Devic	ces		
#1	Primary	1,240.75	5' 24.0" Rour	nd Culvert		
	,	,	L= 31.7' R	CP. square edge	headwall, Ke= 0.500	
				· · · ·	'/ 1,239.70' S= 0.0331 '/' Cc= 0.900	
				low Area= 3.14 s	,	
#2	Device 1	1 240 75	,		e/Grate X 2.00 C= 0.600	
#3	Device 1	,		Orifice/Grate C		
#3	DEVICE	1,241.70			- 0.000	

Primary OutFlow Max=11.53 cfs @ 12.03 hrs HW=1,242.79' TW=1,240.34' (Fixed TW Elev= 1,240.34') **1=Culvert** (Passes 11.53 cfs of 15.44 cfs potential flow)

2=Orifice/Grate (Orifice Controls 6.98 cfs @ 6.20 fps)

-3=Orifice/Grate (Orifice Controls 4.55 cfs @ 3.47 fps)

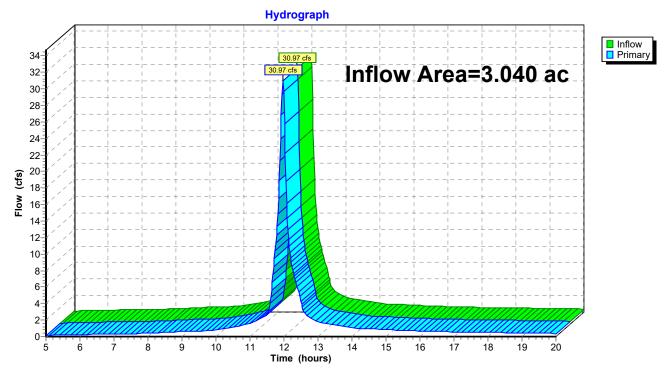




Summary for Link 15L: Link

Inflow Area =		3.040 ac, 49.34% Impervious, Inflow Depth > 7.36" for 100 year event
Inflow	=	30.97 cfs @ 11.97 hrs, Volume= 1.865 af
Primary	=	30.97 cfs @ 11.97 hrs, Volume= 1.865 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.02 hrs



Link 15L: Link